



REPORT

EIMS

Hydrogeological Desktop Assessment for Sandgat Project Area

Submitted to:
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Prepared by:
Hydrogeek Consulting (PTY) Ltd

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Distribution history

| Name | Company |
|---------------|---------|
| Monica Niehof | EIMS |
| | |

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Compiled by: Nico van Zyl (MSc Hydrogeology)

Hydrogeek Consulting Pty Ltd

Email: nico@hydrogeekconsulting.com

Address: 3 Nicholas Cres, Stratford Gardens, Broadacres, 2021

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The findings, results, observations, conclusions, and recommendations presented in this report are based on the author's best scientific and professional knowledge, as well as the available information. The assessment techniques employed are constrained by the information, time, and budgetary limitations relevant to the scope of the investigation. Hydrogeek Consulting (Pty) Ltd; (hereafter referred to as Hydrogeek) reserves the right to modify aspects of the report, including recommendations, should new information emerge from ongoing research, monitoring, or further work in the field.

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This report has been prepared in accordance with the latest requirements for specialist reports set forth by the Department of Environmental Affairs, as outlined in Government Gazette No. 40713 (24 March 2017) and Government Gazette No. 40772 (07 April 2017) under the National Environmental Management Act, 1998 (Act No. 107 of 1998) (NEMA).

I, NL van Zyl, hereby declare that:

-  I act as the independent specialist for this application.
-  I will conduct the work related to this application objectively, even if it leads to findings that are not favorable to the applicant.
-  I have no circumstances that could compromise my objectivity in this work.
-  I possess the necessary expertise relevant to this application, including knowledge of the Act, Regulations, and relevant guidelines.
-  I will comply with all applicable legislation.
-  I have not engaged, nor will I engage, in any conflicting interests related to this activity.
-  I will disclose to the applicant and the competent authority any material information that could influence decisions regarding the application or the objectivity of any report or document prepared for submission.
-  All information I have provided in this declaration is true and correct.

NL VAN ZYL

MSc. Hydrogeology, *Pr.Sci.Nat.*

LIST OF ABBREVIATIONS

| | |
|-------|--|
| DWAF | Department of Water Affairs and Forestry |
| DWS | Department of Water and Sanitation |
| EC | Electrical Conductivity |
| ET | Evapotranspiration |
| K | Hydraulic Conductivity |
| MAP | Mean Annual Precipitation |
| MAR | Mean Annual Runoff |
| NGA | National Groundwater Archive |
| NWA | National Water Act (Act 36 of 1998) |
| S | Storativity |
| T | Transmissivity |
| WARMS | Water Use Authorisation and Registration Management System |
| WMA | Water Management Area |

1 Introduction

EIMS has appointed Hydrogeek Consulting (Pty) Ltd (Hydrogeek) to undertake a groundwater desktop assessment for the delineated study area (Sandgat) associated with potential future prospecting activities. The objective of the assessment is to review and analyse available groundwater-related information to provide an initial understanding of the initial hydrogeological conditions underlying the site, as well as to comment on the groundwater potential and possible impacts underlying the study area.

This desktop assessment provides a preliminary evaluation of the groundwater regime based on existing data sources. The findings will inform the need for further, site-specific hydrogeological investigations and support early-stage decision-making, while ensuring alignment with the requirements of the National Water Act (Act 36 of 1998) and applicable environmental legislation.

2 Scope of Work

This report has been compiled in support of the application for a Prospecting Right and the associated Environmental Authorisation (EA). The purpose of the report is to provide a preliminary hydrogeological assessment of the study area and to evaluate the potential impacts associated with the proposed prospecting activities.

The applicant has indicated that prospecting will be limited to the drilling of approximately 16 boreholes distributed across the project area. No additional activities are currently proposed during the prospecting phase.

Accordingly, this assessment focuses on identifying and evaluating the potential hydrogeological and groundwater-related impacts arising from the drilling of these boreholes. The findings presented herein aim to inform the environmental assessment process, support regulatory decision-making, and ensure that any identified risks are appropriately understood and managed at an early stage.

The scope of work is to provide groundwater specialist services, including the tasks outlined below:

1. Desktop assessment of available geological and hydrogeological data:
 - Review available geological, hydrogeological, climatic, and water use data.
 - Identify key aquifer characteristics and potential recharge zones.
 - Determine existing borehole locations and historical use in the surrounding area.
2. Groundwater potential and quality assessment
 - Evaluate the likelihood of successful groundwater development.
 - Provide an indication of expected yield ranges.
 - Provide insights into aquifer type and estimated sustainable abstraction rates.
 - Comment on expected water quality concerns based on regional data.
3. Preparation of a detailed geohydrological desktop report, signed by a qualified professional geohydrologist
 - Submit a geohydrological report, including methodology, findings, mapping, hydrogeological interpretation, and recommendations.

The assessment was undertaken in accordance with accepted hydrogeological practice and was limited to desktop-based review in line with the agreed scope of work.

3 Methodology

The hydrogeological assessment commenced with a comprehensive desktop study to compile and review relevant geological, hydrogeological, environmental, and land-use information for the study area and surrounding region. Information obtained through the desktop review was analysed and interpreted to inform the findings and conclusions presented in this report. This included an assessment of the underlying aquifer system in terms of groundwater occurrence, aquifer vulnerability, susceptibility to contamination, and overall groundwater resource significance.

In addition, the assessment considered the potential hydrogeological impacts associated with the proposed prospecting activities, including the drilling of approximately 16 prospecting boreholes across the application area. The impact assessment was undertaken in accordance with the Environmental Impact Management Services (Pty) Ltd (EIMS) / National Environmental Management Act, 1998 (Act No. 107 of 1998) and regulations (as amended) (NEMA) Environmental Impact Assessment methodology, which considers the nature, extent, duration, intensity, probability, and significance of identified impacts, as well as the effectiveness of proposed mitigation measures. The detailed impact assessment methodology is attached to and forms part of the broader Environmental Impact Assessment process.

4 Project description

No project details were provided at the time of writing this report for the Sandgat prospecting right project. Only the mineral deposit of the area was provided as shown below in Figure 1 below.

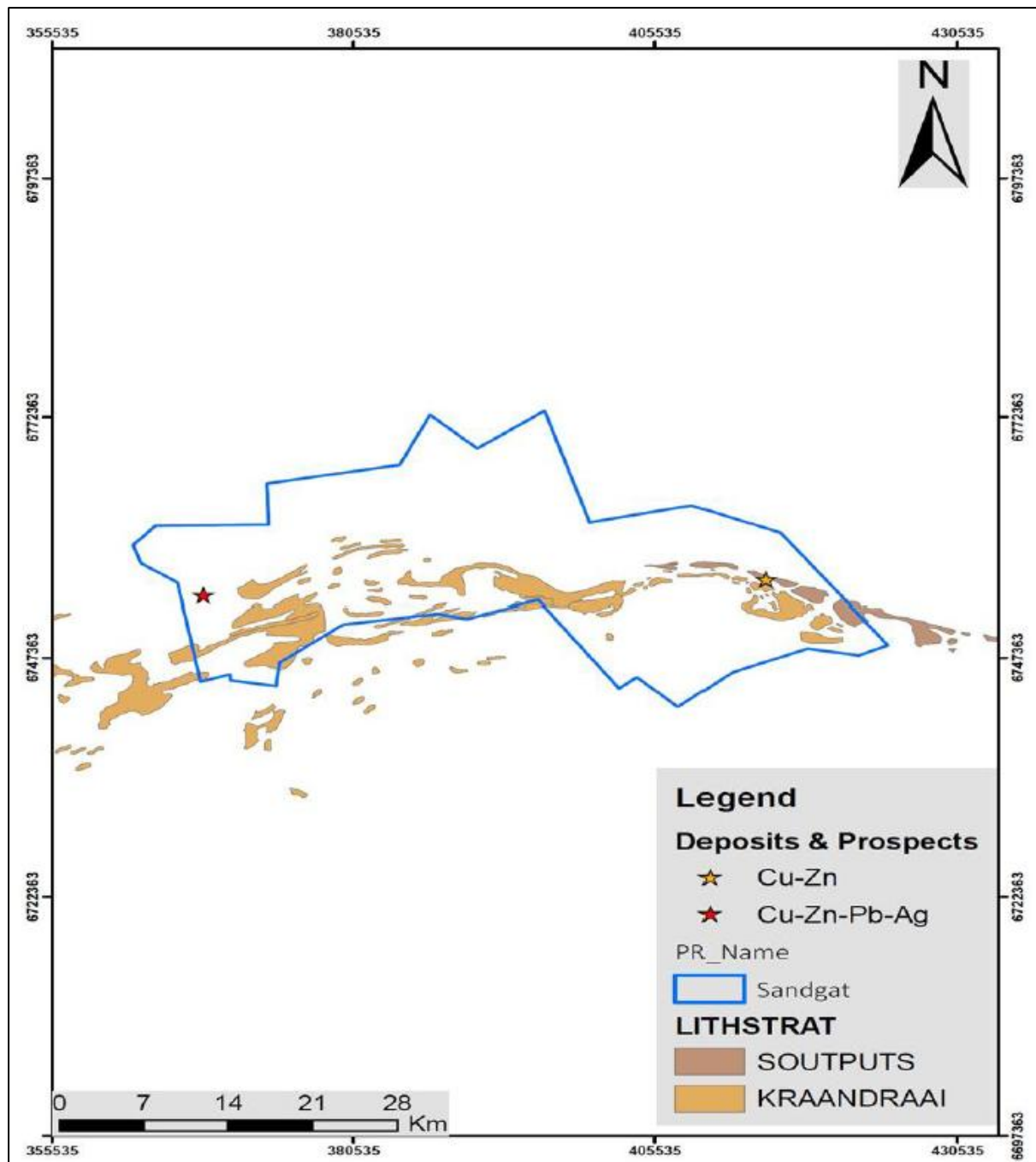


Figure 1: Proposed Project deposits and prospects

Sandgat prospecting right project description has been provided below which list similar activities expected for the project.

Black Mountain Mining (Pty) Ltd (BMM) (the Applicant) has submitted an application for a Prospecting Right in terms of Section 16 of the Mineral and Petroleum Resources Development Act, 2002 (Act 28 of 2002) (MPRDA) and an Application for Environmental Authorisation in terms of Chapter 4 of GNR 982 promulgated under the National Environmental Management Act (Act 107 of 1998) (NEMA) to prospect for ferrous & base metals (Copper Ore, Iron Ore, Lead, Ore, Zinc Ore, Manganese Ore, Nickel and Molybdenum) and associated metals and minerals, precious metals/Gemstones (Gold Ore, Silver Ore), and nuclear fuels/Ferrous and Base metals (Uranium ore) and all associated metals and minerals.

The proposed project will aim to ascertain if economically viable mineral deposits exist within the application area. In order to undertake prospecting activities, Black Mountain Mining will require a Prospecting Right in terms of the Mineral and Petroleum Resources Development Act (MPRDA, Act No.28 of 2002). The Applicant is also required to obtain an Environmental Authorisation (EA) in terms of the National Environmental Management Act (NEMA, Act No. 107 of 1998) which involves the submission of a Basic Assessment Report (BAR) and Environmental Management Programme (EMPr). Environmental Impact Management Services (Pty) Ltd (EIMS) have been appointed by Black Mountain Mining to compile the BAR and EMPr in support of the Prospecting Right application submitted by EIMS on behalf of Black Mountain Mining, which in turn will be submitted to the Department of Mineral and Petroleum Resources (DMPR) for adjudication.

The BAR has been designed to meet the requirements for a BAR and Environmental Management Programme (EMPr) as stipulated in the 2014 EIA Regulations (as amended) promulgated under the NEMA. The adjudicating authority for this Application will be the DMPR, and this report has been compiled in accordance with the applicable DMPR guidelines and reporting template.

This specialist assessment is compiled to support the BAR and EMPr to be submitted.

Details surrounding the project and the type of minerals to be prospected for can be seen in

Table 1: Details of the project and type of minerals

| Item | Detail |
|----------------------------|--|
| Type of mineral(s) | Ferrous & base metals: Cu – Copper |
| Type of minerals continued | Ferrous & base metals: Fe- Iron |
| Type of minerals continued | Ferrous & base metals: Pb -Lead |
| Type of minerals continued | Ferrous & base metals: Zn - Zinc |
| Type of minerals continued | Ferrous & base metals: Mn - Manganese |
| Type of minerals continued | Ferrous & base metals : Ni - Nickel |
| Type of minerals continued | Ferrous & base metals: Mo – Molybdenum |
| Type of minerals continued | Precious Metals: Au - Gold |
| Type of minerals continued | Precious Metals: Ag - Silver |
| Type of minerals continued | Nuclear Fuels: U - Uranium |
| Geological formation | The target geological formation is the Bushmanland Group |

The historic geological data obtained from previous prospecting work in the area and recent prospecting work conducted utilizing advanced geoscientific methods has helped to better refine the underlying geology of the area. This has led to the discovery of Gamsberg East Extension deposit west of this prospecting area and highlighted further potential for the discovery of additional economic zinc-copper-lead-silver mineralization.

This application employs a phased approach, where the work program is divided into several sequential sections. At the end of each section there will be a period of compiling, evaluating and reporting results. These results will not only determine whether the project proceeds, but also the way it will go forward. Essentially, the Applicant will only action the next phase once satisfied with the results obtained.

It is not possible to give details of the drilling program before the surveys and surface work phase 1 is completed. In the event that more information becomes available or that an ore body is located at an earlier stage, then an amended program will be put forward for the DMPR's approval.

No bulk sampling work is to be carried out during this prospecting program.

Initial prospecting will be carried out by the applicant itself, utilizing its own in-house geologists to conduct and oversee the work. Drilling will be outsourced to a local drilling company. The methods detailed below will be used to investigate the prospecting area.

Please note, the "invasive prospecting" phases, which will have an environmental impact, have been highlighted with red text.

Table 2: Proposed duration of prospecting phases and associated activities

| Phase | Activity (what are the activities that are planned to achieve optimal prospecting) | Skill(s) required. (refers to the competent personnel that will be employed to achieve the required results) | Timeframe (in months) for the activity) | Outcome (What is the expected deliverable, e.g., Geological report, analytical results, feasibility study, etc.) | Timeframe for outcome (deadline for the expected outcome to be delivered) | What technical expert will sign off on the outcome? (e.g., geologist, mining engineer, surveyor, economist, etc.) |
|-------|---|---|---|--|---|--|
| 1 | Non-Invasive Prospecting Desktop Study: Literature Survey / Review | Geologist | Month 1-12 | Initial geological targeting report supported by historical records and existing data | Month 12 | Geologist |
| 2 | Non-Invasive Prospecting Geological Field Mapping | Geologist & field crew | Month 6-12 | Detailed geological targeting report accompanied by maps & plans of ground truthing of initial geological targeting. | Month 12 | Geologist |
| 3 | Non-Invasive Prospecting Semi-regional Ground Geophysical Survey | Geophysicist / Geologist / field crew | Month 12-24 | Survey report detailing possible targets for further exploration, report supported by maps, plans & cross sections | Month 24 | Geophysicist |

| Phase | Activity (what are the activities that are planned to achieve optimal prospecting) | Skill(s) required. (refers to the competent personnel that will be employed to achieve the required results) | Timeframe (in months) for the activity) | Outcome (What is the expected deliverable, e.g., Geological report, analytical results, feasibility study, etc.) | Timeframe for outcome (deadline for the expected outcome to be delivered) | What technical expert will sign off on the outcome? (e.g., geologist, mining engineer, surveyor, economist, etc.) |
|-------|--|---|---|--|---|--|
| 4 | Invasive Prospecting Exploration Boreholes (16 RAB holes – 2400m; 4 DD holes – 2000m) | Geologist / drill rig team / field crew / laboratory technicians | Month 24-34 | Borehole cored data & RAB data: lithological logs, geophysical down hole surveys, assay results for mineralized intercepts. | Month 34 | Geologist |
| 5 | Non-Invasive Prospecting Compilation, interpretation, and modeling of data | Geologist / Geophysicist | Month 34-36 | Modelling of data. Interpretation and 3D modeling of potential deposit. Generation & ranking of mineralized targets for further exploration work | Month 36 | Geologist |
| 6 | Non-Invasive Prospecting Detailed Ground Geophysical Survey on individual positively mineralized targets to define extent | Geophysicist / Geologist / field crew | Month 36-42 | Survey report detailing individual targets. Plans for drill hole intersections supported by cross sections | Month 42 | Geophysicist |
| 7 | Invasive Prospecting Boreholes to confirm continuity of mineralization & potential deposit size (20 DD holes – 8000m) | Geologist / drill rig team / field crew / laboratory technicians | Month 42-48 | Widely spaced borehole cored data: lithological logs, geophysical down hole surveys, assay results for mineralized intercepts, metallurgical test work | Month 48 | Geologist |

| Phase | Activity (what are the activities that are planned to achieve optimal prospecting) | Skill(s) required. (refers to the competent personnel that will be employed to achieve the required results) | Timeframe (in months) for the activity) | Outcome (What is the expected deliverable, e.g., Geological report, analytical results, feasibility study, etc.) | Timeframe for outcome (deadline for the expected outcome to be delivered) | What technical expert will sign off on the outcome? (e.g., geologist, mining engineer, surveyor, economist, etc.) |
|-------|---|---|---|--|---|--|
| | | | | Risk assessment study to advance to next phase | | |
| 8 | Invasive Prospecting Resource definition drilling (40 DD holes – 16000m) | Geologist / drill rig team / field crew / laboratory technicians | Month 48-60 | Closely spaced borehole cored data: lithological logs, geophysical down hole surveys, assay results for mineralized intercepts, metallurgical test work Resource estimation work producing an Inferred Mineral Resource | Month 60 | Geologist |
| 9 | Non-Invasive Prospecting Analytical Desktop Pre-Feasibility Study | Economic Geologist / Mining Geologist | Month 54-60 | Geological & Pre-feasibility reports, maps & plans Risk assessment study to determine if full feasibility is warranted | Month 60 | Mine Engineer / Economic Geologist (professionally qualified persons) |

4.1 Description of Planned Invasive Activities

(These activities result in land disturbances e.g., sampling, drilling, bulk sampling, etc.)

4.1.1 Drilling

The targeting of all drilling activities will be dependent on the results obtained during the preceding phases of prospecting, namely the geological mapping and geophysical surveying and as such it is currently not possible to include a finalized surface plan showing the intended location, extent, and depth of boreholes to be completed.

Diamond drilling will be of the standard HQ or NQ size. Down hole surveys will be done every 50m in each hole. Core will be marked, logged, photographed, and sampled according to the standard of the applicant's logging and sampling procedures.

Down the hole geophysical surveying will take place upon completion of the exploratory boreholes along with Ground EM surveys to determine positions of conductors.

Rehabilitation of drill sites will be done according to an approved Environmental Management Plan.

Percussion Rotary Air Blast (RAB) or Reverse Circulation (RC) drilling may be carried out for pre-collaring of diamond drill boreholes or for obtaining samples if significant depth of cover is encountered over particular targets.

4.1.2 Assaying

Rock chip / soil samples will be sent to a laboratory of the applicant's choice to be crushed, split, pulverized, and assayed. Samples from core will be split using a core cutter before being sent to the laboratory for analysis.

4.1.3 Metallurgical Test Work

Metallurgical test work would start during phase 7 of the prospecting work programme. These tests will be done by and in consultation with a preferred and accredited Laboratory of the applicant's choice.

4.1.4 Boreholes

4.1.4.1 Phase 4: Boreholes

The initial planned invasive exploration activities will consist of diamond drill boreholes drilled to appropriate depths to target any anomalies identified during Phases 2 & 3 of the non-invasive portion of the prospecting work plan. The work will consist of:

- ◆ Access and drill site preparation.
- ◆ Diamond core drilling.
- ◆ Sampling and assaying.
- ◆ Quality assurance and quality control programs.
- ◆ Down hole geophysics.
- ◆ Rehabilitation of drill sites; and
- ◆ Recording & Integration of data.

4.1.4.2 Phase 7: Boreholes

This phase of boreholes would determine the continuity of mineralization & potential deposit size. The work will consist of:

- ◆ Access and drill site preparation.
- ◆ Widely spaced diamond drilling and analyses to confirm grade / tonnage potential.

- Sampling and assaying.
- Quality assurance and quality control programs.
- Metallurgical test work.
- Rehabilitation of drill sites; and
- Recording & Integration of data.

4.1.4.3 Phase 8: Boreholes

This phase of boreholes would provide enough information to be able to calculate an inferred resource. The work would consist of:

- Access and drill site preparation.
- Close spaced infill diamond drilling and analyses to determine actual grade / tonnage.
- Sampling and assaying.
- Quality assurance and quality control programs.
- Metallurgical test work.
- Geotechnical drilling program.
- Rehabilitation of drill sites; and
- Recording & Integration of data

5 Study area Location

The study area is located approximately 30 km south-east of the town of Pofadder, within the Northern Cape Province. The boundaries of the site extend across a regional-scale footprint of approximately 47 065 ha and encompasses multiple farm portions and cadastral properties.

The properties included within the study area extent are presented in **Table 3** below:

Table 3: Properties included within the Study area boundary

| Farm Name | Portion | Farm Nr. | Area (ha) |
|------------------|--------------|----------|-----------|
| Lovedale | Remainder | RE/201 | 8 460.99 |
| Ougga Maag | Remainder | RE/200 | 7 660.54 |
| Haartebeest-vlei | Full Portion | 199 | 7 456.8 |
| Vaalkop | Remainder | RE/225 | 8 220.93 |
| Vaalkop | 1 | 1/225 | 0.09 |
| Farm 197 | Remainder | RE/197 | 3 061.62 |
| Farm 197 | 8 | 8/197 | 1 515.12 |
| Farm 197 | 4 | 4/197 | 1 490.51 |
| Farm 197 | 2 | 2/197 | 1 528.65 |
| Farm 197 | 1 | 1/197 | 3 012.34 |
| Farm 197 | 3 | 3/197 | 3 037.83 |
| Farm 197 | 6 | 6/197 | 1 578.93 |

Regional and local maps of the study area location are included in **Figure 2** and **Figure 3**.

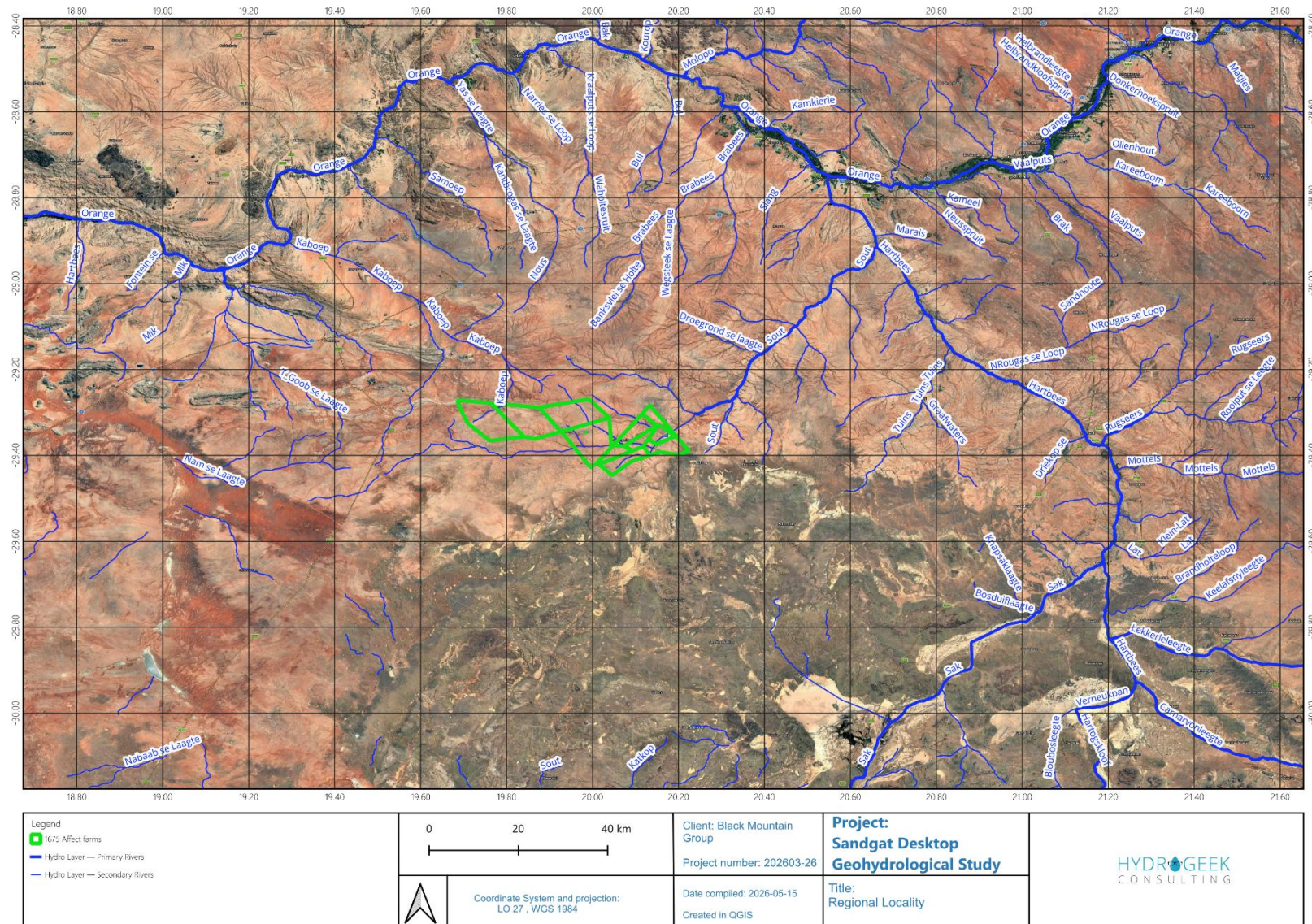


Figure 2: Regional Study area Location (study area), overlaid on satellite imagery.

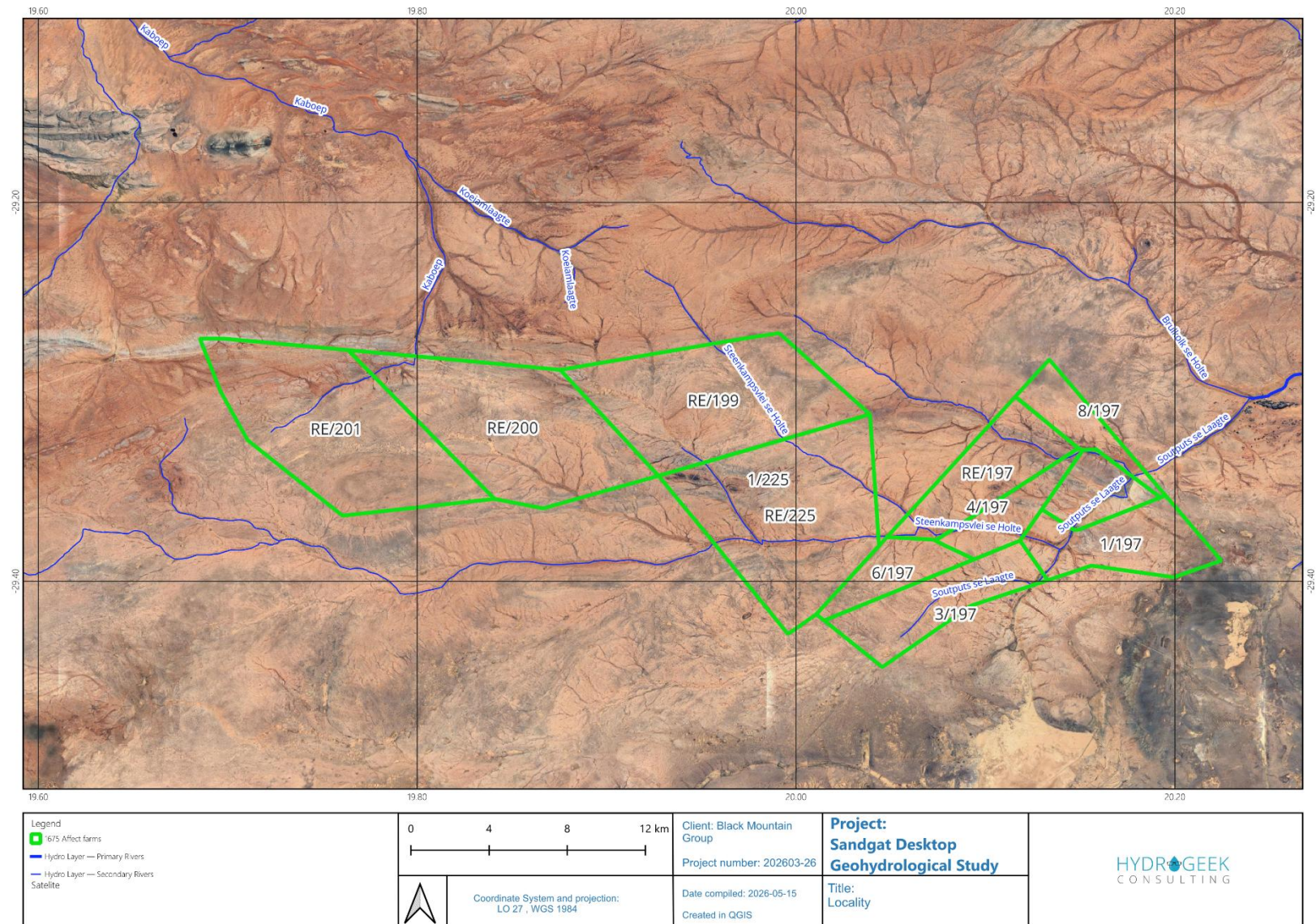


Figure 3: Local Study area Location (study area), overlaid on satellite imagery.

6 Regional Setting

6.1 Climate

The north-western region of the Northern Cape is characterised by a hot, arid to semi-arid climate. Climate data for the town of Pofadder, indicates that rainfall is strongly seasonal, with most precipitation occurring during the summer months (November to March), peaking in February and March. Rainfall events are generally convective in nature, occurring as short-duration, high-intensity storms. Winters (May to August), in contrast, are predominantly dry, with little to no rainfall recorded during this period.

Average rainfall data taken over a 10-year period (2016 to 2026) shows annual rainfall at 84 mm/a (World Weather, 2025). This data also confirms the strongly seasonal nature of precipitation in the area. Average monthly rainfall during the summer months ranges between approximately 9 mm and 24 mm, while winter rainfall is minimal, with averages recorded between 1.6 mm and 5 mm. Pofadder Rainfall data is included in **Figure 4**.

Temperature patterns over the same period show average minimum temperatures during winter dropping to around 6°C, and average maximum temperatures during summer reaching approximately 35°C (World Weather, 2025).

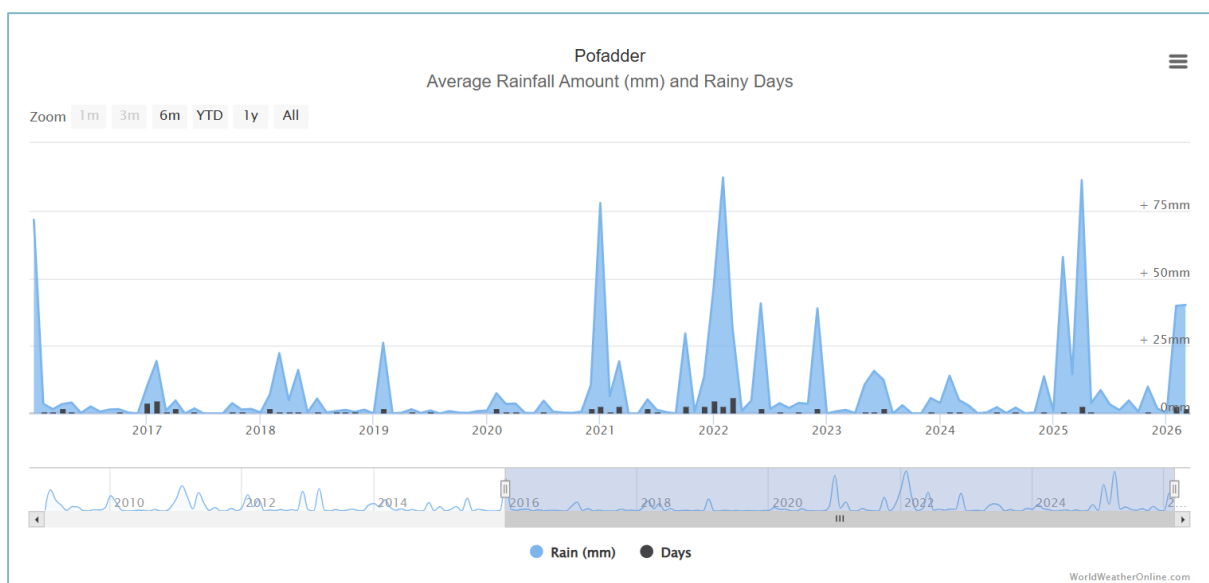


Figure 4: Pofadder Rainfall Data (2016-2026) (source: World Weather Online, 2025).

Evaporation data for the quaternary catchments (D81F and D53G) are derived from the WR2005 dataset, which provides hydrological parameters at quaternary scale based on WRSM/Pitman modelling (WRC, 2012; Middleton and Bailey, 2011). The data indicates very high evaporation of approximately 2 000 mm/a, far exceeding mean annual rainfall. According to DWAF, (2004), only 1 to 10% of rainfall contributes to recharge during larger rainfall events, with most water lost to evaporation.

From a hydrogeological perspective, the area is therefore characterised by low recharge and high evaporative demand. Recharge is largely episodic and event-driven, while surface water systems remain

predominantly ephemeral. Groundwater resources are therefore expected to be limited, with recharge largely episodic and long-term abstraction potential likely constrained.

6.2 Topography and Drainage

The Study area crosses over two (3) quaternary catchments, namely D81F, D81G and D53G which form part of the Lower Orange Water Management Area (WMA). This WMA is dominated by the Orange River, located approximately 55 km north-east of the Study area. The Orange River is the primary river system in the region, and flows westward, ultimately discharging into the Atlantic Ocean.

Table 4 Catchments affected

| Catchment | Quaternary Catchment | Area (km ²) | MAP (mm/a) | MAR (mm/a) | MAR (Mm ³ /a) | Hydrological Zone | Drainage System |
|-----------|----------------------|-------------------------|------------|------------|--------------------------|-------------------|-----------------|
| D81F | D81F | 1,839 | 91.34 | 0.5 | 0.92 | L | Orange River |
| D81G | D81G | 2,005 | 101.64 | 0.4 | 0.80 | L | Orange River |
| D53G | D53G | 4,745 | 98.56 | 0.5 | 2.37 | L | Orange River |

Notably, most tributaries within the WMA (including the Molopo, Kuruman, Sak, and Hartbeest Rivers) are ephemeral, with flow occurring only in response to rainfall events. Hydrological processes within the WMA are therefore largely driven by episodic rainfall, with smaller drainage features being non-perennial in nature (DWAF, 2004; DWS, 2013).

At a local scale, the Study area is traversed by a network of ephemeral drainage features, including the Kaboep River, Steenkampsvlei se Holte, and Soutputs se Laagte. These drainage features follow the natural topography and typically flow only during, and shortly after, rainfall events. As is typical of arid environments, these features are not expected to sustain meaningful baseflow contributions (DWAF, 2004; WRC, 2012).

Topographically, elevations across the study area range from approximately 705 to 1 129 metres above mean sea level (mamsl), with higher elevations located to the west and lower-lying areas to the north-west and south-east. The study area is situated along a local topographic divide, with surface runoff and near-surface runoff expected to move toward the northern and eastern portions of the study area, in line with the prevailing gradient.

Overall, the drainage network is characterised by low flow frequency and high variability, consistent with the arid climatic conditions of the region. Regional topography and drainage patterns are presented in **Figure 5**.

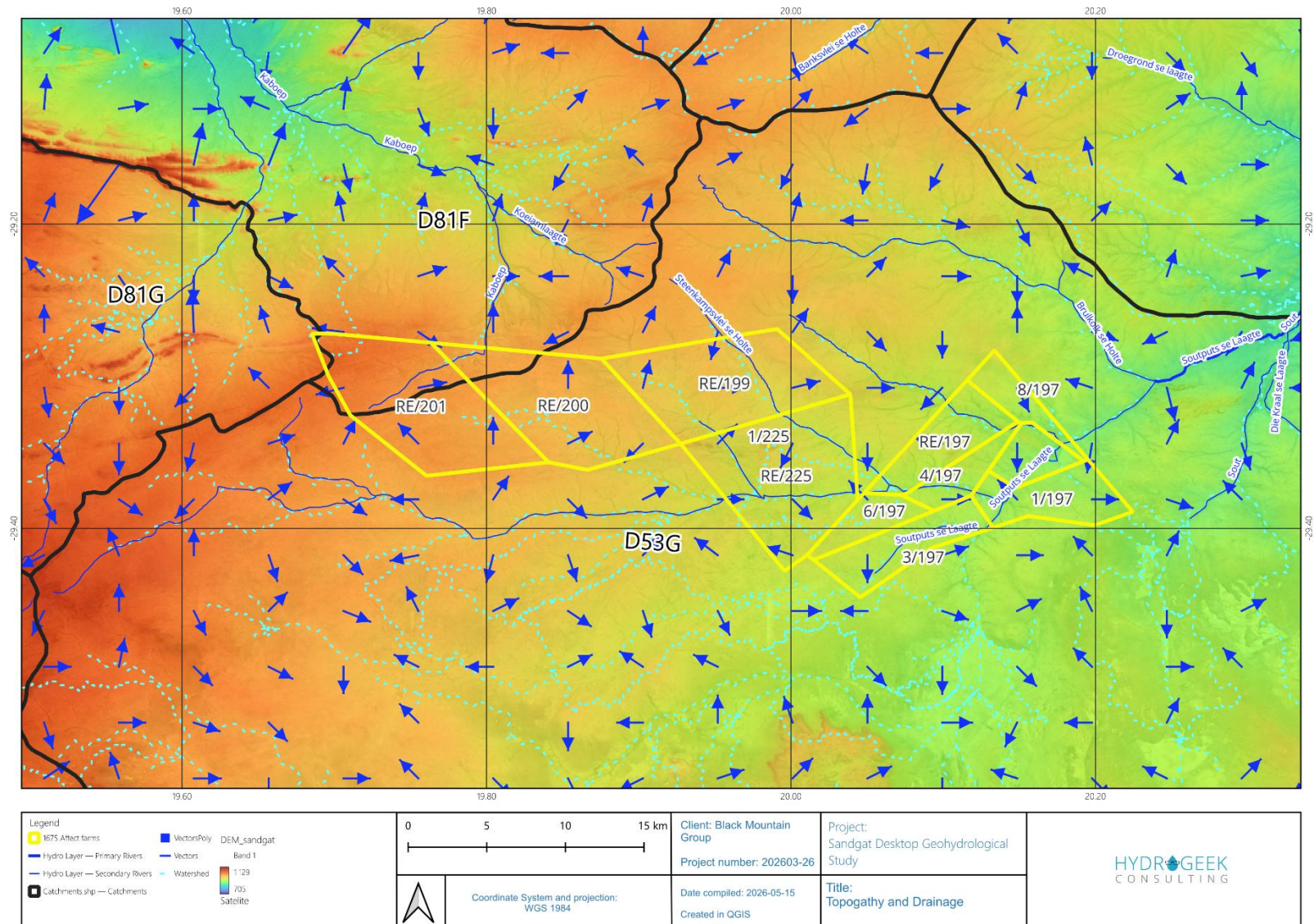


Figure 5: Regional topography and drainage patterns.

6.3 Geology

6.3.1 Regional Geology

According to the 1:250 000 geological map series 2919 Pofadder and 2920 Kenhardt (Council for Geoscience, 1992) (Agenbacht, 2007), the study area is situated within the Namaqualand Metamorphic Complex, which comprises a structurally complex assemblage of gneisses, schists and quartzites. These rocks have undergone multiple phases of deformation and form the regional crystalline basement (Maclaren, 1984; Agenbacht, 2007). The geological setting is strongly influenced by regional deformation and intrusive activity associated with the broader Pofadder Lineament.

The mapped geology includes a sequence of supracrustal meta-sedimentary units, represented by the Brulkolk Formation, Kraandraai Formation, Soutpans Formation and the Droëboom Group. These units consist predominantly of quartzites, schists and quartz-feldspar gneisses, with minor amphibolitic and pelitic lithologies (Maclaren, 1984; Agenbacht, 2007). The rocks are typically well indurated and exhibit negligible primary porosity.

The Kamiesberg Group comprises high-grade metamorphic rocks, including gneisses, amphibolites and schists, and forms part of the deeper crystalline basement. These units have been subjected to polyphase deformation, resulting in a complex structural fabric (Maclaren, 1984; Agenbacht, 2007; Viljoen et al., 2010).

The basement rocks are intruded by granitic to granitoid bodies, including units mapped as the T'oubep Suite, emplaced during regional tectonic events (Agenbacht, 2007). Earlier intrusive phases include mafic and intermediate rocks described by Maclaren (1984), such as the Nouzees Gabbroic Suite and associated granitoid intrusions. In places, the basement rocks are overlain by sediments of the Dwyka Group (Karoo Supergroup), comprising glacial diamictites (tillites) with minor interbedded sediments (Maclaren, 1984; Agenbacht, 2007).

The area is further affected by post-tectonic dolerite intrusions, occurring as dykes and sills, most likely associated with Karoo-age magmatism (Maclaren, 1984; Agenbacht, 2007). The emplacement of these intrusions appears to be structurally controlled and associated with pre-existing zones of weakness.

The study area is traversed by a prominent structural corridor, interpreted as a shear zone associated with the regional Pofadder Lineament (Maclaren, 1984; Agenbacht, 2007). This zone is discussed in detail in **Section 6.3.2**, and is characterised by folding, faulting and shearing, and represents a zone of mechanical weakness within the bedrock.

6.3.2 Structural

As briefly discussed, the study area is influenced by major shear zones, including the Nouzees shear zone, which play an important role in controlling both the emplacement of intrusive bodies and the distribution of secondary permeability. The study area is traversed by a prominent structural corridor, interpreted as a shear zone associated with the regional Pofadder Lineament. This zone is characterised by folding, faulting and shearing, and represents a zone of mechanical weakness within the bedrock.

Geology in the region has undergone multiple phases of deformation associated with the Namaqua Orogeny. Four principal deformation events (**F1-F4**) have been identified (Jackson, 1982).

The **F1** phase is characterised by tight, high-amplitude folding, most clearly developed in well-banded rocks. This was followed by the **F2** event, which represents the peak of regional metamorphism. During this phase, the rocks were further deformed under high temperature and pressure, resulting in a range of fold styles and the development of a strong, penetrative foliation (Jackson, 1982).

The **F3** phase is associated with flexural slip folding, where the rocks folded while also shearing along existing foliation planes. This resulted in the formation of broader, more open structures that generally trend east-west. The final phase, **F4**, is less prominent in the area, but where present, is characterised by north-west trending, gently curved (monoclinal) structures with steep eastern limbs (Jackson, 1982). Pegmatites are commonly linked to this phase and typically occur within folds or along associated fractures.

Later deformation led to the development of north-west trending fault zones, particularly to the north of Pofadder. Movement along these faults was predominantly horizontal movement, with the rocks shifting to the right, past one another. In places, this movement caused intense deformation, resulting in intensely sheared and deformed rock (Jackson, 1982).

Groundwater occurrence is largely controlled by these structural features, reflecting the limited primary porosity of the bedrock and resulting in generally low and variable yields.

The regional geology is depicted in **Figure 6**.

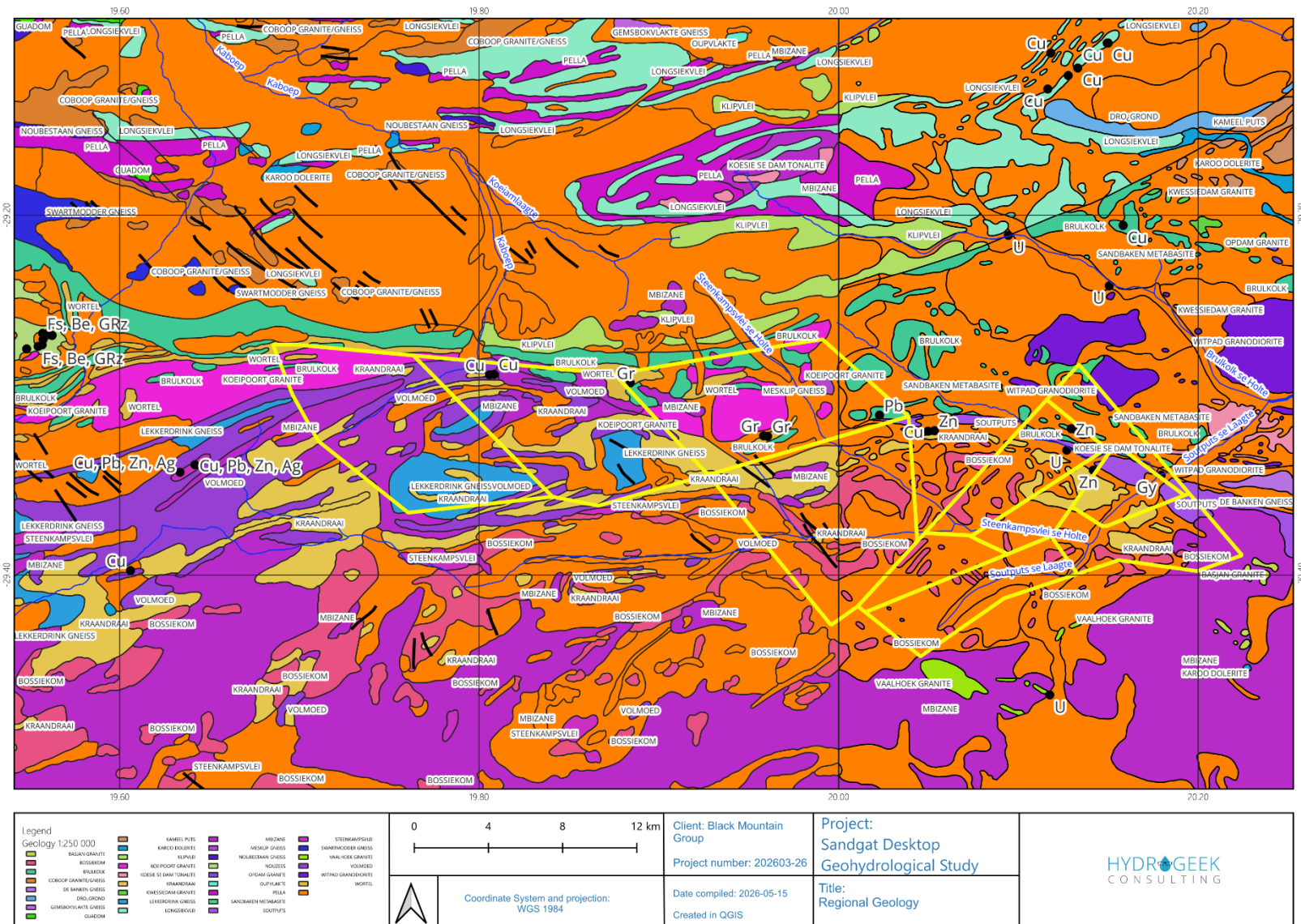


Figure 6: Regional Geology

7 Hydrogeology

From a hydrogeological perspective, the bedrock units within the study area are characterised by low primary porosity, with groundwater occurrence restricted to secondary features such as fractures, joints, foliation planes and shear zones (Viljoen et al., 2010). Groundwater movement is structurally controlled, with shear zones and well-developed fracture networks representing the most significant conduits due to their enhanced permeability and connectivity. In particular, groundwater is closely associated with fracturing and shearing within the metamorphic basement, as well as weathered zones within gneisses and schists.

The presence of major shear zones associated with the Pofadder Lineament is therefore considered to be the primary control on groundwater occurrence within the study area. These structurally weakened zones represent the most favourable targets for groundwater development, owing to increased fracturing and hydraulic connectivity. In contrast, the surrounding metamorphic and granitic units are expected to exhibit limited groundwater potential unless intersected by such structural features.

The overlying sediments of the Dwyka Group generally comprise low permeability diamictites and are not expected to constitute significant aquifers. Similarly, dolerite intrusions may locally influence groundwater flow, either acting as barriers to movement or, where sufficiently fractured, enhancing permeability.

Table 5 Hydrogeological units

| Group / Suite | Subgroup / Formation | Lithology | Hydrogeological Characteristics | Aquifer Type | Groundwater Potential | Comments |
|--|------------------------------------|--------------------------------|---|-------------------------------|--------------------------------|---|
| Namaqua Metamorphic Complex | Gneissic Units | Banded gneiss, granitic gneiss | Very low primary porosity; groundwater limited to weathered and fractured zones | Fractured / weathered aquifer | Low to moderate | Regionally extensive basement; yields depend on degree of weathering and fracturing |
| Bushmanland Group (or equivalent supracrustal units) | Quartzite Units | Quartzite, meta-sandstone | Moderate to high secondary permeability due to fracturing | Fractured aquifer | Moderate to high | Primary aquifer target; best yields along fractures and contacts |
| Bushmanland Group (or equivalent) | Schist Units | Mica schist, foliated rocks | Anisotropic permeability along foliation planes | Fractured anisotropic aquifer | Low to moderate | Controls flow direction; may act as conduit or barrier depending on orientation |
| Younger Sedimentary Cover (if present) | Undifferentiated Sedimentary Units | Sandstone/Shale | Variable permeability; locally intergranular storage | Intergranular + fractured | Low to moderate | May act as semi-confined aquifer or aquitard depending on composition |
| Dwyka (Karoo Supergroup) | Tillite | Diamictite (glacial deposits) | Very low permeability | Aquitard | Very low | Acts as barrier; may create perched groundwater conditions |
| Karoo Suite | Dolerite Dykes/Sills | Dolerite | Low matrix permeability; enhanced flow along fractures and contacts | Fractured aquifer | Low to moderate (locally high) | Structurally important; contact zones may enhance yields |

Table 6 Expected Aquifer Characteristics

| Parameter | Expected Range / Behaviour |
|------------------------|---|
| Aquifer Type | Fractured basement |
| Hydraulic Conductivity | Highly variable (10^{-6} – 10^{-3} m/s locally in fractures) |
| Yield | Low to moderate, highly variable |
| Storage | Low (unless weathered zone present) |
| Recharge | Low, episodic |
| Flow regime | Structurally controlled |

7.1 Aquifer Yield

According to the 1:500 000 scale hydrogeological map (DWAF, 2000), the study area is predominantly underlain by meta-calcareous rocks (calc-silicate gneisses). These rocks typically occur as banded units, often interlayered with quartzite and schist. Groundwater occurrence is largely dependent on secondary permeability, associated with fractures and shear zones. **Figure 7** depicts the relative site position overlaid on the Springbok 2916- hydrogeological map.

The study area is therefore characterised predominantly by a fractured aquifer system, with localised occurrences of intergranular and fractured aquifers toward the western extent of the site. Based on the 1:500 000 scale hydrogeological map, both aquifer types are associated with regional average borehole yield indications of 0.1 to 0.5 L/s. **Figure 8** depicts the principal groundwater occurrence of the study area.

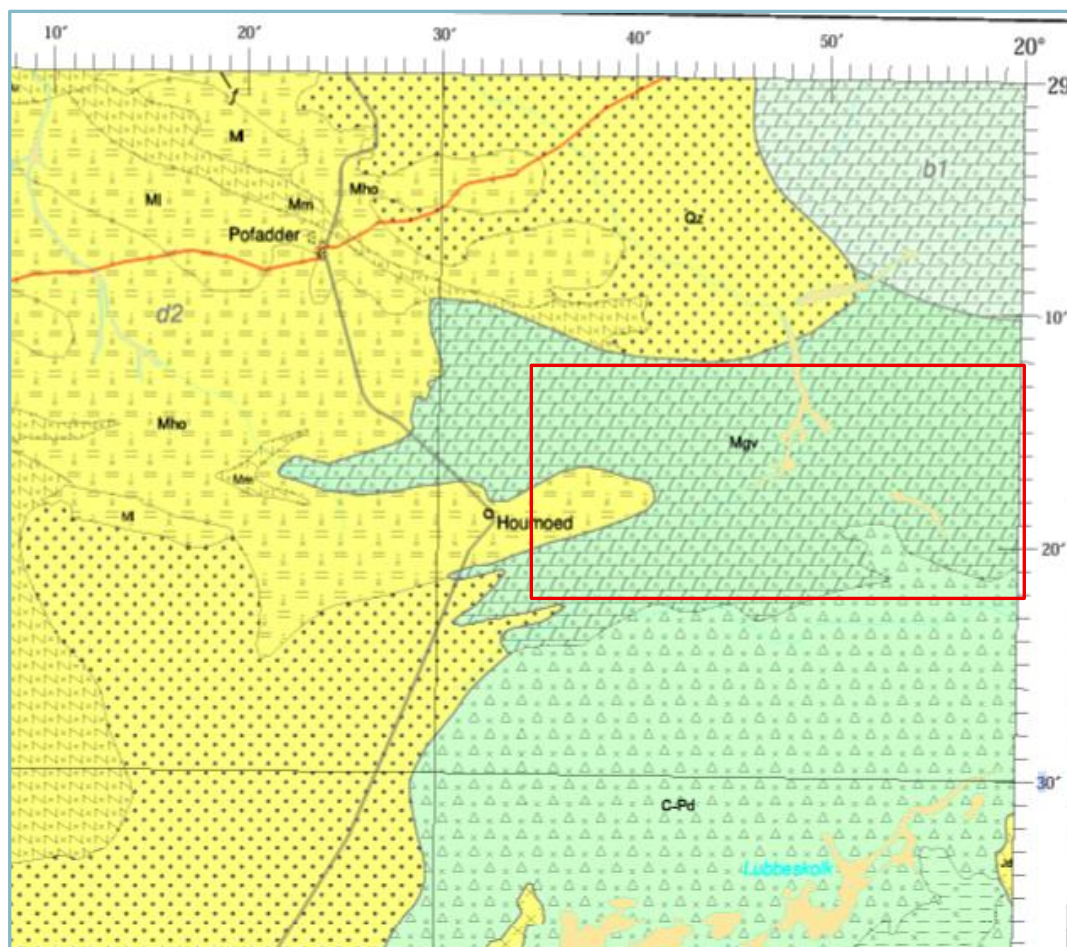


Figure 7: 1:500 000 scale hydrogeological map Springbok 2916 (DWAF, 2000)

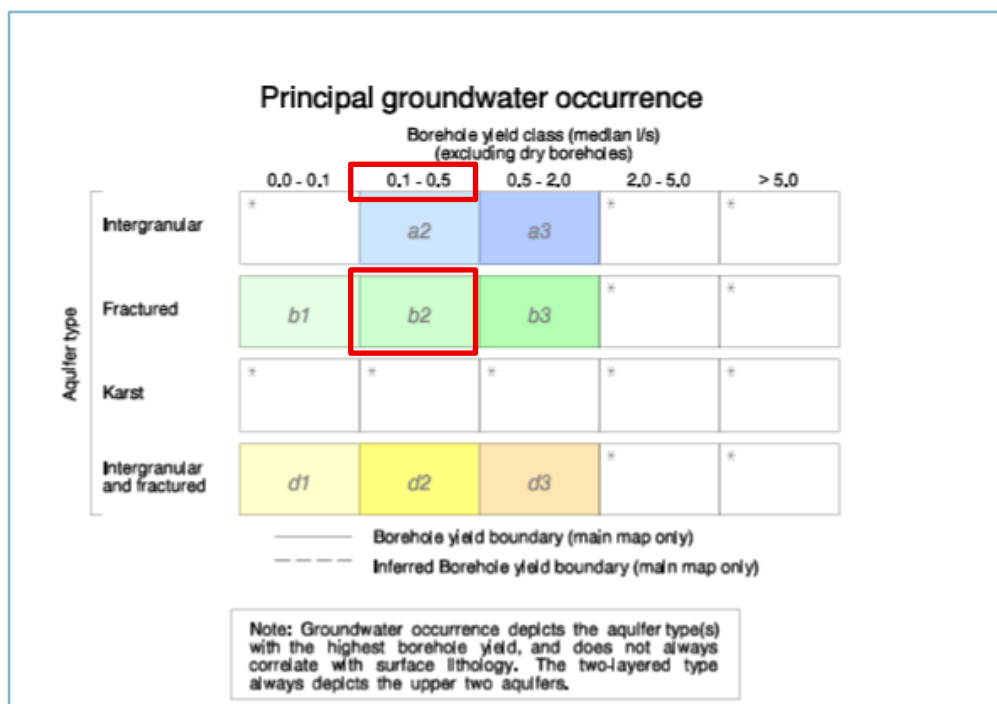


Figure 8: 1:500 000 scale hydrogeological map Springbok 2916, Principal Groundwater Occurrence (DWAF, 2000)

7.2 Aquifer Recharge

As part of the desk study, the Vegter Groundwater Recharge Map (Vegter, 1995) provides a regional context of natural groundwater recharge potential. This map, developed for South Africa, uses long-term climatic, geological, and hydrological data to estimate average annual recharge rates across the country. Recharge values are expressed in mm/yr of mean annual precipitation (MAP) and are influenced by factors such as rainfall intensity, evapotranspiration, soil type, vegetation cover, and underlying geology.

Recharge estimates by Vegter indicate values in the order of 0.1 to 3 mm/a for the study area. Groundwater recharge estimates derived from the Groundwater Resource Assessment Phase 2 (GRA2) similarly indicate extremely low recharge, with values approaching negligible levels (0 mm/yr) within the area (DWS, 2005). Taking this into account, the recharge is estimated to range between approximately 0% and 3.6% of the average mean annual precipitation (84 mm/yr). Such low recharge values are considered typical for arid environments. In addition, high evaporation rates characteristic of the region further reduces the proportion of rainfall contributing to groundwater recharge.

While the Vegter recharge map is useful for broad-scale assessments and comparative analysis between regions, it is intended as a regional-scale tool. Study area-specific recharge rates may differ significantly due to localised conditions such as land use changes, aquifer properties, and artificial recharge or abstraction impacts. Consequently, the map is best used as a screening-level indicator during the scoping phase, with study area-specific measurements and modelling recommended for accurate recharge estimation. **Figure 9** depicts the relative position of the study area relative to the estimated recharge.

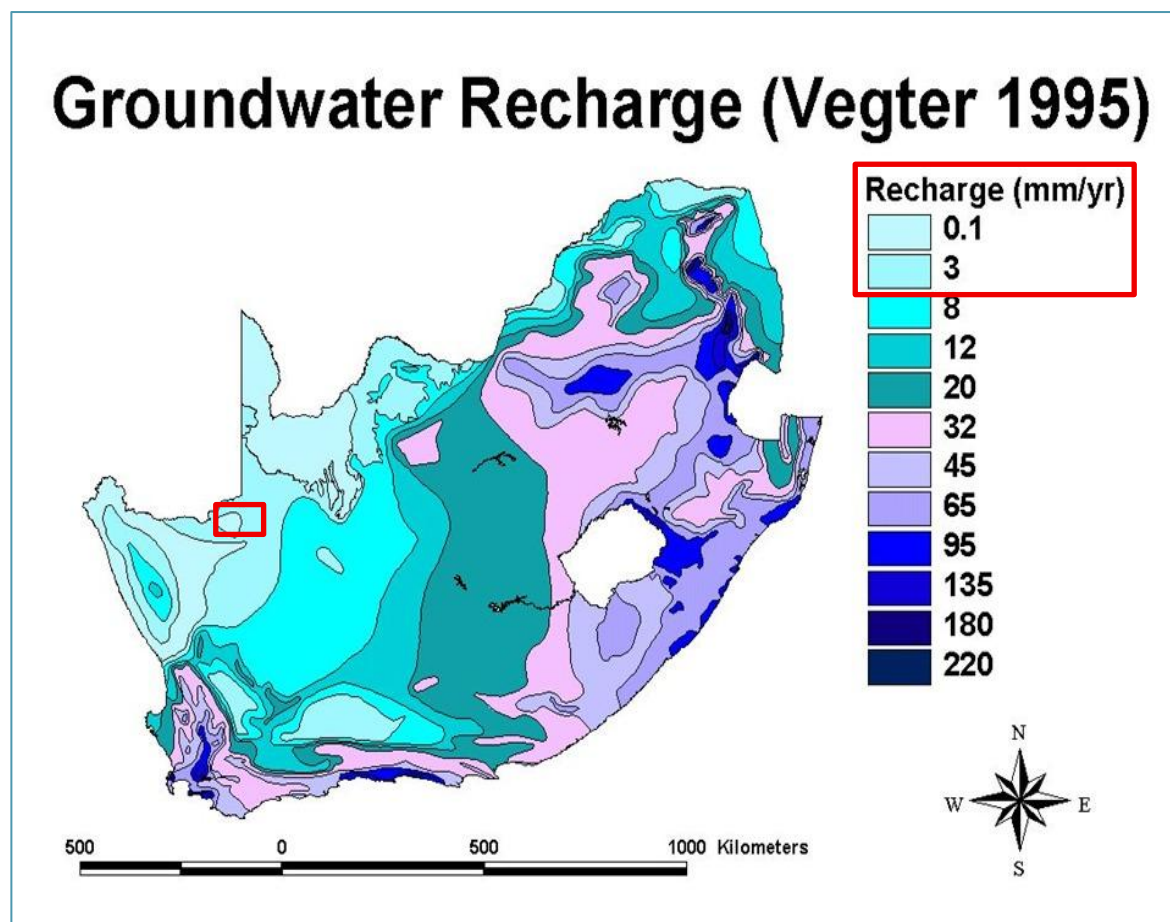


Figure 9 Vegter, 1995, Recharge Map

7.3 Aquifer Quality

Electrical Conductivity (EC) reflects the groundwater's ability to conduct an electrical current and is directly proportional to the concentration of dissolved ions present in the water. It serves as a useful indicator of groundwater quality.

According to the 1:500 000 scale groundwater quality map (DWA, 2012), EC values range between 150 and 370 mS/m in the western extent of the study area, corresponding to 'acceptable' groundwater quality. These values increase to approximately 370 to 520 mS/m across the central and eastern portions, indicative of 'poor' (brackish) groundwater quality.

To more accurately characterise groundwater quality within the local area, groundwater samples should be collected from any available boreholes or water points within and around the Study area and submitted for laboratory analysis. If no boreholes are currently present, the installation of monitoring boreholes should be considered to enable representative sampling.

In addition, isotopic analysis may be considered to assess potential groundwater-surface water interactions, particularly in relation to the non-perennial drainage lines. Such analyses would assist in evaluating the degree of hydraulic connectivity and the potential impacts of activities such as dewatering on the local surface water system, and support any potential water use licensing process.

7.4 Groundwater flow directions

In natural, undeveloped settings, groundwater flow generally follows the hydraulic gradient from areas of higher groundwater head toward areas of lower head, which often broadly reflects surface topography. In fractured hard-rock environments, however, groundwater flow may also be significantly influenced by geological and structural controls such as fractures, faults, foliation planes, and shear zones. As a result, groundwater flow patterns do not necessarily mirror surface drainage or topographic gradients.

Within the study area, groundwater flow is therefore expected to be influenced by a combination of topography and structural features. Given the fractured nature of the underlying bedrock, structurally controlled flow along preferential pathways such as fractures and shear zones associated with the regional west-east Pofadder Lineament is deemed likely. Local drainage features may also act as zones of focused recharge or discharge during and after rainfall events, particularly where these intersect with structurally weakened zones.

At this stage, however, the available desktop information is insufficient to determine site-specific groundwater flow directions with confidence. No representative set of static groundwater level measurements is available across the study area to define groundwater heads or hydraulic gradients, and the current interpretation should therefore be regarded as conceptual only. To improve accuracy in the inferred groundwater flow direction, static groundwater levels should be measured across the study area and surrounding area during any future field-based hydrogeological assessment.

7.5 Aquifer Vulnerability Classification

7.5.1 Aquifer Classification

The underlying aquifer was classified in accordance with the South African Aquifer System Management Classification framework developed by Parsons (1995), which continues to serve as the national standard for assessing the significance of groundwater resources. This framework offers a strategic basis for the sustainable management and protection of aquifer systems by considering their hydrogeological properties, yield potential, and socio-economic value.

Based on the available data, the aquifer beneath the study area is classified as a Minor Aquifer. Such aquifers generally exhibit moderate yields sufficient for localised water supply but are not regarded as significant on a regional scale. Nevertheless, they warrant protection in cases of local dependency or heightened environmental sensitivity.

A summary of the aquifer system classification scheme and associated definitions is presented in Table 7.

Table 7: Aquifer System Management Classes (Parsons, 1995)

| | |
|----------------------|--|
| Sole source aquifer | An aquifer which is used to supply 50% or more of domestic water for a given area, and for which there are no reasonable available alternative sources should the aquifer be impacted upon or depleted. Aquifer yields and natural water quality are immaterial. |
| Major aquifer system | Highly permeable formations, usually with a known probable presence of significant fracturing. They may be highly productive and able to support large abstractions for public supply and other purposes. Water quality is generally very good (less than 150 mS/m). |

| | |
|------------------------|---|
| Minor aquifer system | These can be fractured or potentially fractured rocks, which do not have a high primary permeability, or other formations of variable permeability. Although these aquifers seldom produce large quantities of water, they are important both for local supplies and supplying base flow to rivers. |
| Non aquifer system | These are formations with negligible permeability that are generally regarded as not containing groundwater in exploitable quantities. Water quality may also be such that it renders the aquifer as unusable. However, groundwater flow through such rocks, although imperceptible, does take place, and needs to be considered when assessing the risk associated with persistent pollutants. |
| Special aquifer system | An aquifer designated as such by the Minister of Water Affairs, after due process. |

7.5.2 Aquifer vulnerability

Aquifer vulnerability refers to the intrinsic susceptibility of a groundwater system to contamination, specifically the likelihood that pollutants introduced at or near the land surface will reach a defined position within the aquifer. This concept considers the natural protective capacity of overlying geo-logical layers, recharge rates, and the hydraulic properties of the aquifer material.

According to the National Aquifer Vulnerability Map of South Africa (DWS, 2013), the aquifer system underlying the project area is classified as having Least Vulnerability. This rating indicates that the aquifer is only vulnerable to conservative pollutants in the long term when continuously discharged or leached.

7.5.3 Aquifer susceptibility

Aquifer susceptibility is a qualitative indicator of the relative ease with which a groundwater system may become contaminated as a result of human activities. It reflects the sensitivity of the aquifer to potential pollution based on factors such as depth to water table, recharge potential, lithology, and land use.

As per the National Aquifer Susceptibility Map of South Africa (DWS, 2013), the aquifer underlying the project area is assigned a Low Susceptibility rating. This suggests a low potential for contamination from surface or near-surface sources, highlighting the importance of implementing appropriate management and mitigation measures to safeguard groundwater quality.

7.6 Aquifer Protection

7.6.1 GQM Index

The Groundwater Quality Management (GQM) Index, developed by the Water Research Commission (WRC) and the former Department of Water Affairs and Forestry (DWAF) in 1995, forms part of South Africa's national framework for aquifer system management classification. Despite its name, the index does not assess water quality, but rather the management importance of groundwater resources based on aquifer characteristics, yield potential, and socio-economic reliance.

The GQM Index supports the strategic categorisation of aquifers and informs decisions related to groundwater protection, development, and monitoring priorities. It remains a key tool in implementing sustainable

groundwater management under the National Water Act (Act 36 of 1998). Based on the GQM Index, the aquifer underlying the Study area requires Low Level Protection.

The Aquifer Vulnerability Classification is depicted in Table 8, with the GQM Index depicted in Table 9, below.

Table 8: Aquifer Vulnerability Classification (WRC & DWAF, 1995)

| Aquifer system | | Aquifer vulnerability | |
|----------------------------|--------|-----------------------|--------|
| Management qualification | | Classification | |
| Class | Points | Class | Points |
| Sole Source Aquifer System | 6 | High | 3 |
| Major Aquifer System | 4 | Medium | 2 |
| Minor Aquifer System | 2 | Low | 1 |
| Non-Aquifer System | 0 | | |
| Special Aquifer System | 0-6 | | |

Table 9: Groundwater Quality Management Index (WRC & DWAF, 1995)

| GQM INDEX | Level of protection |
|-----------|---------------------------|
| <1 | Limited Protection |
| 1 to 3 | Low Level Protection |
| 3 to 6 | Medium Level Protection |
| 6 to 10 | High Level Protection |
| >10 | Strictly Non- Degradation |

A study area-specific aquifer vulnerability assessment using the widely applied DRASTIC index (which considers factors such as water table depth, aquifer media, and recharge) can be incorporated in more detailed studies. However, this requires field-based data collection, including a hydrocensus, which was not conducted as part of the current investigation.

8 Site Specific Information

To determine whether there are any groundwater users in the area that may be affected by activities within the study area, a database search was conducted around the property boundary. This portion of the study was completed by studying and reviewing existing national databases that contain groundwater information. A search was conducted on a number of databases, namely the National Groundwater Archive (NGA) and the Water Use Authorisation and Registration Management System (WARMS). These resources provide data on borehole positions, groundwater chemistry and yield, when available.

8.1 National Groundwater Archive (NGA) Database

An assessment of the NGA database, incorporating information on borehole locations, groundwater quality, and yields, identified a total of 84 boreholes located within the study area boundary and a 1 km radius of the study area. A summary of data provided is in Appendix A.

A limited dataset of three (3) groundwater level measurements, recorded between 1990 and 1996, indicates water table depths ranging from 15.00 mbgl to 45.00 mbgl. The limited lithological descriptions captured in the NGA dataset suggest deeper water levels associated with quartzitic formations compared to more shallow granitic settings.

A discharge of approximately 2 L/s was recorded for borehole 2919BD00030, which is recorded as being drilled into quartzite. This value likely represents a reported pumping rate rather than a confirmed sustainable yield, as no supporting pumping test data are available. As such, it should be considered indicative only.

NGA data obtained for groundwater levels and discharge in and proximal to the study area is depicted in Table 10.

Table 10: NGA Data on Groundwater Levels and Discharge Rates

| Identifier | Latitude (DD, WGS84) | Longitude (DD, WGS84) | Water Level Measure- ment Date | Water Level (mBGL) | Discharge Rate Meas- urement Date | Dis- charge Rate | Depth Diame- ter Measure- ment Date | Depth to Bot- tom | Casing Depth to Bottom | Water Strike Depth to Top | Lithol- ogy Depth to Bottom | Lithology Name |
|-------------|----------------------------|-----------------------------|--|--------------------------|--|------------------------|---|----------------------------|------------------------------|------------------------------------|--------------------------------------|-------------------|
| 2919BD00032 | -29.336469 | 19.776430 | 1996/04/24 | 31 | No data | No data | 24/04/1996 | 6.00 | 6 | 31.00 | 72.00 | Quartzite |
| | | | | | | | | | | 56.00 | | |
| | | | | | | | | | | 31.00 | | |
| | | | | | | | | 72.00 | | 56.00 | | |
| 2919BD00030 | -29.296189 | 19.888100 | 1990/08/02 | 45 | 02/08/1990 | 2.000 l/s | 02/08/1990 | 6.60 | 7 | 60.00 | 66.00 | Quartzite |
| | | | | | | | | 60.00 | | | | |
| 2920AC00013 | -29.431190 | 20.000650 | 1990/11/06 | 15 | No data | No data | 06/11/1990 | 30.00 | 6 | | 30.00 | Granite |



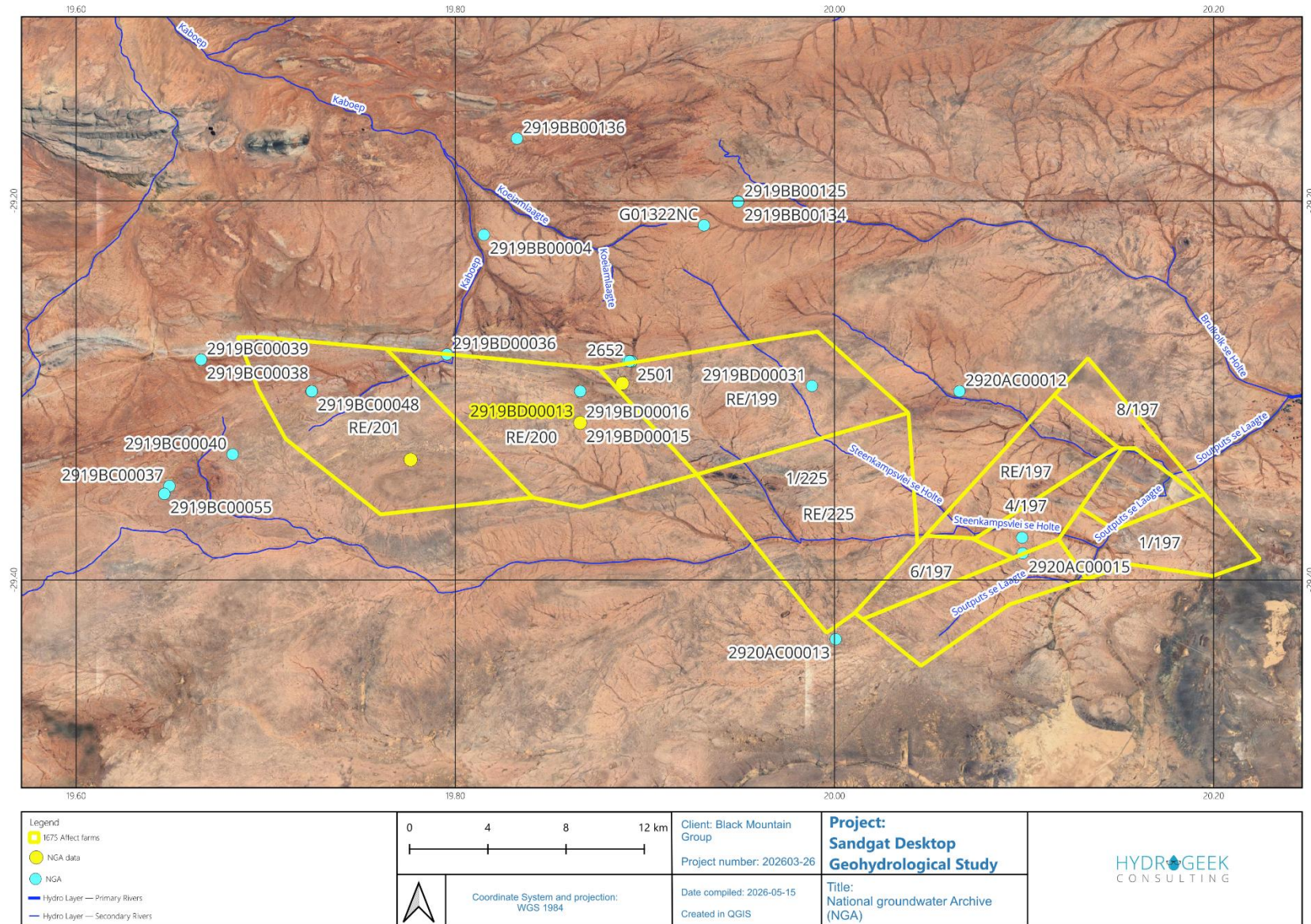


Figure 10 National Groundwater Archive (NGA) boreholes



8.2 Water Use Authorisation and Registration Management System (WARMS) Database

The WARMS is a national database managed by the Department of Water and Sanitation (DWS), used to record and monitor registered water uses across South Africa in accordance with the National Water Act (Act 36 of 1998). The system includes data on licensed and authorised water users, volumes abstracted, and types of water use. For this study, available WARMS data from 2025 was obtained from DWS and used to assess the status of registered water use within the surrounding area of the study area.

Assessment of the WARMS Database showed no active WARMS registrations in proximity to the study area. The nearest WARMS point (registered water user) is situated 5 km north of the study area boundary. Information on this point is included in Table 11 below.

Table 11: Summary of nearest WARMS registered water use point.*

| Study area ID | Latitude (DD, WGS84) | Longitude (DD, WGS84) | Registered Volume (m ³ /a) | Use | Status |
|---------------|----------------------|-----------------------|---------------------------------------|-------------------------|--------|
| 25023466 | -29.1928 | 19.7679 | 66 000 | Agriculture: Irrigation | Lawful |

**It should be noted that the accuracy of information obtained from the database could not be independently verified and may contain errors.*

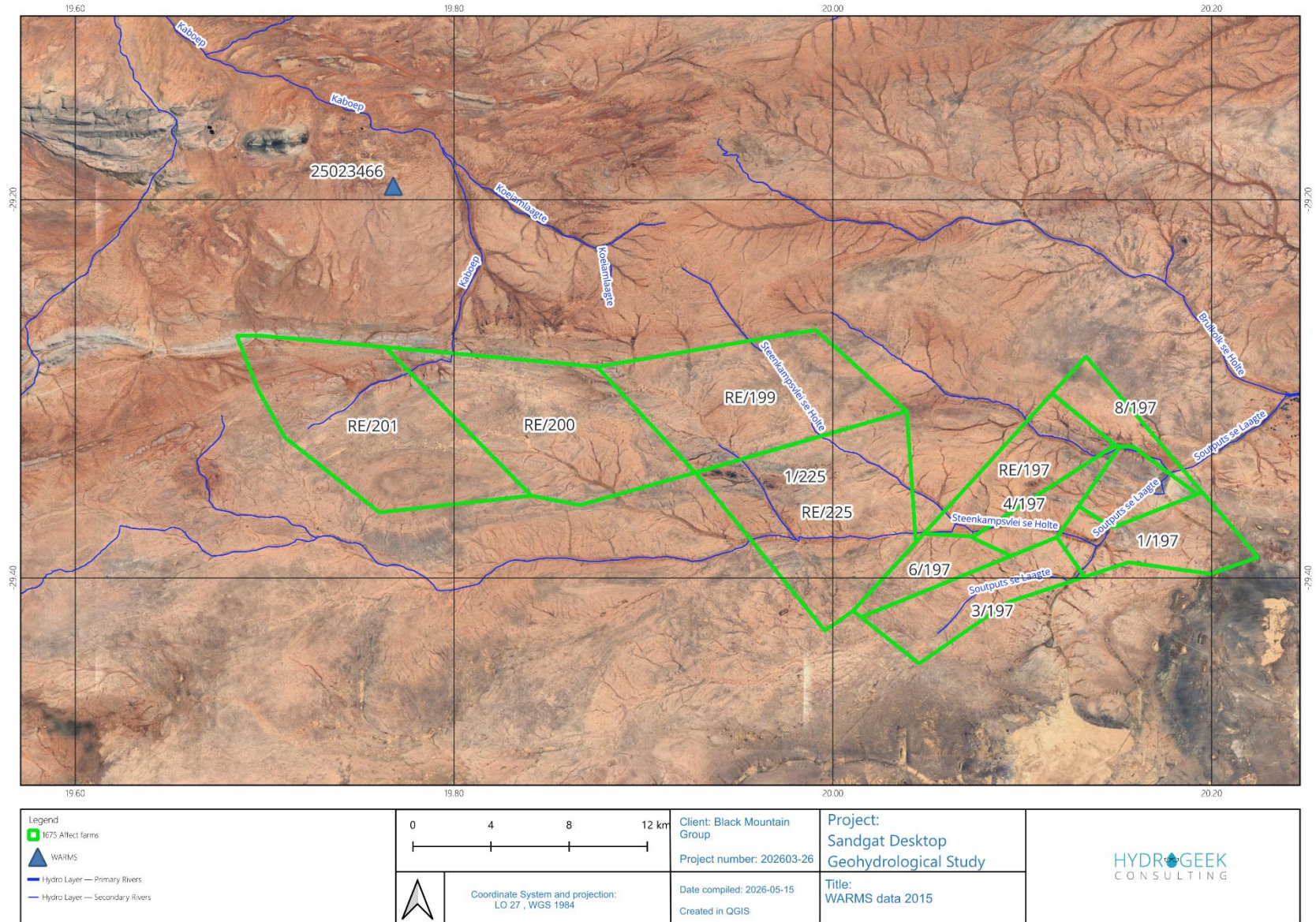


Figure 11 Water Use Authorisation and Registration Management System (WARMS) data

9 HYDROGEOLOGICAL RISK ASSESSMENT AND ENVIRONMENTAL MANAGEMENT PROGRAMME

The study considered the potential cumulative impacts associated with the proposed prospecting programme, including the drilling of approximately 16 prospecting boreholes across the application area. The cumulative assessment evaluated the potential combined effects of drilling activities on groundwater resources, aquifer integrity, groundwater quality, and surrounding environmental receptors, considering existing activities within the broader area.

9.1 Desktop level

Statement on Impact Differentiation

The assessment included a desktop sensitivity assessment of the study area in relation to the proposed prospecting activities and associated infrastructure, specifically limited to the drilling of approximately 16 prospecting boreholes for sampling and exploration purposes during the construction/prospecting phase. The assessment considered the geological and hydrogeological setting, aquifer vulnerability, groundwater resource significance, surface water features, drainage characteristics, potential environmentally sensitive areas, and existing land-use activities within and surrounding the application area.

Areas considered environmentally sensitive, including drainage features, wetlands, watercourses, steep slopes, and areas with elevated groundwater vulnerability, were identified as areas to be avoided where reasonably possible during the planning and positioning of prospecting boreholes and associated temporary access routes. The findings of the desktop assessment indicate that the proposed prospecting activities are localised, temporary in nature, and of relatively limited hydrogeological impact, provided that appropriate management and mitigation measures are implemented.

Potential impacts associated with the prospecting drilling activities during the construction/prospecting phase may include

- Localised disturbance of surface soils,
- Potential contamination of groundwater through poor drilling practices
- hydrocarbon spillages
- Improper handling of drilling fluids and cuttings
- Limited temporary disturbance to shallow groundwater flow conditions

No significant long-term impacts on regional groundwater resources are anticipated as part of the prospecting phase, as no abstraction, dewatering, or mining-related excavation forms part of the current application

9.1.1 Mitigation and management measures.

- Boreholes should be positioned outside of sensitive drainage features, wetlands, and watercourses where reasonably possible.
- All drilling activities should be undertaken using appropriate drilling and environmental management practices to prevent groundwater contamination.
- Hydrocarbon fuels, oils, and hazardous substances should be stored in designated bunded areas away from drainage lines and boreholes.
- Spill prevention and spill response measures should be implemented on site at all times.
- Drill sites should be kept clean, and all waste materials and drilling-related waste should be removed and disposed of at approved facilities.
- Disturbed areas should be rehabilitated following completion of drilling activities.
- Boreholes not required for future monitoring or exploration purposes should be appropriately sealed and decommissioned in accordance with recognised standards.
- No uncontrolled discharge of contaminated water or drilling fluids should be permitted to the surrounding environment.

9.2 Risk Assessment Criteria

9.2.1 Procedure

The impact significance rating methodology, as presented herein and utilised for all EIMS Impact Assessment Projects, is guided by the requirements of the NEMA EIA Regulations 2014 (as amended). The approach may be altered or substituted on a case by case basis if the specific aspect being assessed requires such—such instances require prior EIMS Project Manager approval. The broad approach to the significance rating methodology is to determine the significance (S) of an environmental risk or impact by considering the consequence (C) of each impact (comprising Nature, Extent, Duration, Magnitude, and Reversibility) and relating this to the probability/ likelihood (P) of the impact occurring. The S is determined for the pre- and post-mitigation scenario. In addition, other factors, including cumulative impacts and potential for irreplaceable loss of resources, are used to determine a prioritisation factor (PF) which is applied to the S to determine the overall final significance rating (FS). The impact assessment will be applied to all identified alternatives.

9.2.2 Determination of Significance

The final significance (FS) of an impact or risk is determined by applying a prioritisation factor (PF) to the post-mitigation environmental significance. The significance is dependent on the consequence (C) of the particular impact and the probability (P) of the impact occurring. Consequence is determined through the consideration of the Nature (N), Extent (E), Duration (D), Magnitude (M), and Reversibility (R) applicable to the specific impact.

For the purpose of this methodology the consequence of the impact is represented by:

$$C = \frac{(E + D + M + R) * N}{4}$$

Each individual aspect in the determination of the consequence is represented by a rating scale as defined in Table 12 below.

Table 12 Criteria for Determining Impact Consequence

| Aspect | Score | Definition |
|---------------------------------|-------|---|
| Nature | - 1 | Likely to result in a negative/ detrimental impact |
| | +1 | Likely to result in a positive/ beneficial impact |
| Extent | 1 | Activity (i.e. Highly localised, limited to the area applicable to the specific activity) |
| | 2 | Site (i.e. within the development property or site boundary, or the area within a few hundred meters of the site) |
| | 3 | Local (i.e. beyond the site boundary within the Local administrative boundary (e.g. Local Municipality) or within consistent local geographical features, or the area within 5 km of the site) |
| | 4 | Regional (i.e. Far beyond the site boundary, beyond the Local administrative boundaries within the Regional administrative boundaries (e.g. District Municipality), or extends into different distinct geographical features, or extends between 5 and 50 km from the site). |
| | 5 | Provincial / National / International (i.e. extends into numerous distinct geographical features, or extends beyond 50 km from the site). |
| Duration | 1 | Immediate (<1 year, quickly reversible) |
| | 2 | Short term (1-5 years, less than project lifespan) |
| | 3 | Medium term (6-15 years) |
| | 4 | Long term (15-65 years, the impact will cease after the operational life span of the project) |
| | 5 | Permanent (>65 years, no mitigation measure of natural process will reduce the impact after construction/ operation/ decommissioning). |
| Magnitude/ Intensity | 1 | Minor (where the impact affects the environment in such a way that natural, cultural and social functions and processes are not affected) |
| | 2 | Low (where the impact affects the environment in such a way that natural, cultural and social functions and processes are slightly affected, or affected environmental components are already degraded) |
| | 3 | Moderate (where the affected environment is altered but natural, cultural and social functions and processes continue albeit in a modified way; moderate improvement for +ve impacts; or where change affects area of potential conservation or other value, or use of resources). |
| | 4 | High (where natural, cultural or social functions or processes are altered to the extent that it will temporarily cease; high improvement for +ve impacts; or where change affects high conservation value areas or species of conservation concern) |
| | 5 | Very high / don't know (where natural, cultural or social functions or processes are altered to the extent that it will permanently cease, substantial improvement for +ve impacts; or disturbance to pristine areas of critical conservation value or critically endangered species) |

| | | |
|----------------------------|---|---|
| Revers- ibility | 1 | Impact is reversible without any time and cost. |
| | 2 | Impact is reversible without incurring significant time and cost. |
| | 3 | Impact is reversible only by incurring significant time and cost. |
| | 4 | Impact is reversible only by incurring very high time and cost. |
| | 5 | Irreversible Impact. |

Once the C has been determined, the significance is determined in accordance with the standard risk assessment relationship by multiplying the C and the P. Probability is rated/ scored as per Table 13.

It is noted that both environmental risks as well as environmental impacts should be identified and assessed. Environmental Risk can be regarded as the potential for something harmful to happen to the environment, and in many instances is not regarded as something that is expected to occur during normal operations or events (e.g. unplanned fuel or oil spills at a construction site). Probability and likelihood are key determinants or variables of environmental risk. Environmental Impact can be regarded as the actual effect or change that happens to the environment because of an activity and is typically an effect that is expected from normal operations or events (e.g. vegetation clearance from site development results in loss of species of concern). Typically the probability of an unmitigated environmental impact is regarded as highly likely or certain (management and mitigation measures would ideally aim to reduce this likelihood where possible). In summary, environmental risk is about what could happen, while environmental impact is about what does happen.

Table 13: Probability/ Likelihood Scoring

| | | |
|--------------------|---|---|
| Probability | 1 | Improbable (Rare, the event may occur only in exceptional circumstances, the possibility of the impact materialising is very low as a result of design, historic experience, or implementation of adequate corrective actions; <5% chance). |
| | 2 | Low probability (Unlikely, impact could occur but not realistically expected; >5% and <20% chance). |
| | 3 | Medium probability (Possible, the impact may occur; >20% and <50% chance). |
| | 4 | High probability (Likely, it is most probable that the impact will occur- > 50 and <90% chance). |
| | 5 | Definite (Almost certain, the impact is expected to, or will, occur, >90% chance). |

The result is a qualitative representation of relative significance associated with the impact. Significance is therefore calculated as follows:

$$S = C \times P$$

Table 14 Determination of Significance

| | | | | | | |
|-------------|--------------|---------------|--------|------------------------|------------------------|------------------------------------|
| Consequence | 5- Very High | 5 | 10 | 15 | 20 | 25 |
| | 4- High | 4 | 8 | 12 | 16 | 20 |
| | 3- Medium | 3 | 6 | 9 | 12 | 15 |
| | 2- Low | 2 | 4 | 6 | 8 | 10 |
| | 1- Very low | 1 | 2 | 3 | 4 | 5 |
| | | 1- Improbable | 2- Low | 3- Medium/ Possible | 4- High/ Prob- able | 5- Highly likely/ Defi- nite |
| Probability | | | | | | |

The outcome of the significance assessment will result in a range of scores, ranging from 1 through to 25. These significance scores are then grouped into respective classes as described in Table 15

Table 15 Significance Scores

| S Score | Description |
|--------------|--|
| ≤4.25 | Low (i.e. where this impact is unlikely to be a significant environmental risk/ reward). |
| >4.25, ≤8.5 | Low-Medium (i.e. where the impact could have a significant environmental risk/ reward). |
| >8.5, ≤13.75 | High-Medium (i.e. where the impact could have a significant environmental risk/ reward). |
| >13.75 | High (i.e. where the impact will have a significant environmental risk/ reward). |

The impact significance will be determined for each impact without relevant management and mitigation measures (pre-mitigation significance), as well as post implementation of relevant management and mitigation measures (post-mitigation significance). This allows for a prediction in the degree to which the impact can be managed/mitigated.

9.2.3 Impact Prioritization

Further to the assessment criteria presented in the section above, it is necessary to consider each potentially significant impact in terms of:

- Cumulative impacts; and
- The degree to which the impact may cause irreplaceable loss of resources.

To ensure that these factors are considered, an impact prioritisation factor (PF) will be applied to each impacts' post-mitigation significance (post-mitigation). This prioritisation factor does not aim to detract from the

significance ratings but rather to focus the attention of the decision-making authority on the higher priority/significance issues and impacts. The PF will be applied to the post-mitigation significance based on the assumption that relevant suggested management/mitigation impacts are implemented.

Table 16: Criteria for Determining Prioritisation

| | | |
|---|------------|--|
| Cumulative Impact (CI) | Low (1) | Considering the potential incremental, interactive, sequential, and synergistic cumulative impacts, it is unlikely that the impact will result in spatial and temporal cumulative change. |
| | Medium (2) | Considering the potential incremental, interactive, sequential, and synergistic cumulative impacts, it is probable that the impact will result in spatial and temporal cumulative change. |
| | High (3) | Considering the potential incremental, interactive, sequential, and synergistic cumulative impacts, it is highly probable/ definite that the impact will result in spatial and temporal cumulative change. |
| Irreplaceable Loss of Resources (LR) | Low (1) | Where the impact is unlikely to result in irreplaceable loss of resources. |
| | Medium (2) | Where the impact may result in the irreplaceable loss (cannot be replaced or substituted) of resources but the value (services and/or functions) of these resources is limited. |
| | High (3) | Where the impact may result in the irreplaceable loss of resources of high value (services and/or functions). |

The value for the final impact priority is represented as a single consolidated priority, determined as the sum of each individual criteria represented in Table 16. The impact priority is therefore determined as follows:

$$\text{Priority} = CI + LR$$

The result is a priority score which ranges from 2 to 6 and a consequent PF ranging from 1 to 1.5 (Refer to Table 17).

Table 17: Determination of Prioritisation Factor

| Priority | Prioritisation Factor |
|----------|-----------------------|
| 2 | 1 |
| 3 | 1.125 |
| 4 | 1.25 |

| Priority | Prioritisation Factor |
|----------|-----------------------|
| 5 | 1.375 |
| 6 | 1.5 |

In order to determine the final impact significance (FS), the PF is multiplied by the post-mitigation significance scoring. The ultimate aim of the PF is an attempt to increase the post mitigation environmental risk rating by a factor of 0.5, if all the priority attributes are high (i.e. if an impact comes out with a high medium environmental risk after the conventional impact rating, but there is significant cumulative impact potential and significant potential for irreplaceable loss of resources, then the net result would be to upscale the impact to a higher significance).

Table 18: Final Environmental Significance Rating

| Significance Rating | Description |
|---------------------|--|
| <-25 | Very High (Impacts in this class are extremely significant and pose a very high environmental risk. In certain instances these may represent a fatal flaw. They are likely to have a major influence on the decision and may be difficult or impossible to mitigate. Offset's may be necessary). |
| <-13.75 to -25 | High negative (These impacts are significant and must be carefully considered in the decision-making process. They have a high environmental risk or impact and require extensive mitigation measures). |
| -8.5 to -13.75 | Medium-High negative (i.e. Impacts in this class are more substantial and could have a significant environmental risk. They may influence the decision to develop in the area and require more robust mitigation measures). |
| <-4.25 to <-8.5 | Medium- Low negative (i.e. These impacts are slightly more significant than low impacts but still do not pose a major environmental risk. They might require some mitigation measures but are generally manageable). |
| -1 to -4.25 | Low negative (i.e. Impacts in this class are minor and unlikely to have a significant environmental risk. They do not influence the decision to develop in the area and are typically easily mitigated). |
| 0 | No impact |
| 1 to 4.25 | Low positive |
| >4.25 to <8.5 | Medium-Low positive |
| 8.5 to 13.75 | Medium-High positive |

| Significance Rating | Description |
|------------------------|---------------|
| >13.75 | High positive |

The significance ratings and additional considerations applied to each impact will be used to provide a quantitative comparative assessment of the alternatives being considered. In addition, professional expertise and opinion of the specialists and the environmental consultants will be applied to provide a qualitative comparison of the alternatives under consideration. This process will identify the best alternative for the proposed project.

9.2.4 Cumulative impact

The cumulative impacts associated with the drilling of sixteen prospecting boreholes are anticipated to be localised, temporary, and of low significance following the implementation of appropriate mitigation measures. Potential cumulative impacts include minor disturbance of surface soils and vegetation, temporary alteration of shallow groundwater flow conditions, and a low risk of groundwater contamination associated with drilling activities, hydrocarbon spillages, and improper handling of drilling fluids and cuttings. Due to the limited number, small footprint, and temporary nature of the proposed boreholes, significant cumulative impacts on groundwater resources or surrounding environmental features are not expected. With adherence to recognised drilling standards, spill prevention measures, and appropriate rehabilitation and sealing of boreholes after completion, the overall cumulative impact is of low negative significance.

9.3 EMPR inclusion

See attached Table 21

Table 19 Risk Assessment Rating Consequence

| Impacts (Construction and Operation) | Phase | Extent | Duration | Magnitude | Reversibility | Nature | Consequence | Probability | Pre-mitigation Significance | Description | Mitigation measure |
|--|---------|--------|----------|-----------|---------------|--------|-------------|-------------|-----------------------------|-------------|---|
| Localised disturbance of surface soils, | Desktop | 2 | 1 | 2 | 2 | -1 | -1,75 | 2 | -3,5 | Low | Boreholes should be positioned outside of sensitive drainage features, wetlands, and watercourses where reasonably possible. |
| Potential contamination of groundwater through poor drilling practices | Desktop | 2 | 1 | 2 | 2 | -1 | -1,75 | 2 | -3,5 | Low | All drilling activities should be undertaken using appropriate drilling and environmental management practices to prevent groundwater contamination. |
| Hydrocarbon spillages | Desktop | 2 | 1 | 2 | 2 | -1 | -1,75 | 2 | -3,5 | Low | Hydrocarbon fuels, oils, and hazardous substances should be stored in designated bunded areas away from drainage lines and boreholes. |
| Improper handling of drilling fluids and cuttings | Desktop | 2 | 1 | 2 | 2 | -1 | -1,75 | 2 | -3,5 | Low | Spill prevention and spill response measures should be implemented on site at all times. |
| Limited temporary disturbance to shallow groundwater flow conditions | Desktop | 2 | 1 | 2 | 2 | -1 | -1,75 | 2 | -3,5 | Low | Disturbed areas should be rehabilitated following completion of drilling activities. Boreholes not required for future monitoring or exploration purposes should be appropriately sealed and decommissioned in accordance with recognised standards |
| Impacts with Mitigation (Decommissioning) | Phase | Extent | Duration | Magnitude | Reversibility | Nature | Consequence | Probability | Significance | | |
| Localised disturbance of surface soils, | Desktop | 2 | 1 | 1 | 1 | -1 | -1,25 | 2 | -2,5 | Low | Boreholes should be positioned outside of sensitive drainage features, wetlands, and watercourses where reasonably possible. |
| Potential contamination of groundwater through poor drilling practices | Desktop | 2 | 1 | 1 | 1 | -1 | -1,25 | 2 | -2,5 | Low | All drilling activities should be undertaken using appropriate drilling and environmental management practices to prevent groundwater contamination. |

| Impacts (Construction and Operation) | Phase | Extent | Duration | Magnitude | Reversibility | Nature | Consequence | Probability | Pre-mitigation Significance | Description | Mitigation measure |
|--|---------|--------|----------|-----------|---------------|--------|-------------|-------------|-----------------------------|-------------|---|
| Hydrocarbon spillages | Desktop | 2 | 1 | 1 | 1 | -1 | -1,25 | 2 | -2,5 | Low | Hydrocarbon fuels, oils, and hazardous substances should be stored in designated bunded areas away from drainage lines and boreholes. |
| Improper handling of drilling fluids and cuttings | Desktop | 2 | 1 | 1 | 1 | -1 | -1,25 | 2 | -2,5 | Low | Spill prevention and spill response measures should be implemented on site at all times. |
| Limited temporary disturbance to shallow groundwater flow conditions | Desktop | 2 | 1 | 1 | 1 | -1 | -1,25 | 2 | -2,5 | Low | Disturbed areas should be rehabilitated following completion of drilling activities. Boreholes not required for future monitoring or exploration purposes should be appropriately sealed and decommissioned in accordance with recognised standards |

Table 20 Risk Assessment Rating Final Environmental Significance

| Impacts | Cumulative Impact (CI) | Loss of Resources (LR) | Priority (P) | Priority factor (PF) | Post mitigation Significance | Final Significance (FS) | | Mitigation measure |
|--|------------------------|------------------------|--------------|----------------------|------------------------------|-------------------------|--------------|---|
| Localised disturbance of surface soils, | 1 | 1 | 2 | 1 | -2,5 | -2,5 | Low negative | Boreholes should be positioned outside of sensitive drainage features, wetlands, and watercourses where reasonably possible. |
| Potential contamination of groundwater through poor drilling practices | 1 | 1 | 2 | 1 | -2,5 | -2,5 | Low negative | All drilling activities should be undertaken using appropriate drilling and environmental management practices to prevent groundwater contamination. |
| Hydrocarbon spillages | 1 | 1 | 2 | 1 | -2,5 | -2,5 | Low negative | Hydrocarbon fuels, oils, and hazardous substances should be stored in designated bunded areas away from drainage lines and boreholes. |
| Improper handling of drilling fluids and cuttings | 1 | 1 | 2 | 1 | -2,5 | -2,5 | Low negative | Spill prevention and spill response measures should be implemented on site at all times. |
| Limited temporary disturbance to shallow groundwater flow conditions | 1 | 1 | 2 | 1 | -2,5 | -2,5 | Low negative | Disturbed areas should be rehabilitated following completion of drilling activities. Boreholes not required for future monitoring or exploration purposes should be appropriately sealed and decommissioned in accordance with recognised standards |

Table 21 EMPr Inclusion

| Mitigation Measures | Phase | Timeframe | Responsible Party for Implementation | Monitoring Party (Frequency) | Target | Performance Indicators (Monitoring Tool) |
|--|---------|-----------------------|--------------------------------------|------------------------------|---|--|
| Boreholes should be positioned outside of sensitive drainage features, wetlands, and watercourses where reasonably possible. | Desktop | Prior to construction | Applicant | One off | Ensure compliance with relevant legislation | No legal directives Legal compliance audit scores |
| All drilling activities should be undertaken using appropriate drilling and environmental management practices to prevent groundwater contamination. | Desktop | Prior to construction | Applicant | One off | Ensure compliance with relevant legislation | No legal directives Legal compliance audit scores |
| Hydrocarbon fuels, oils, and hazardous substances should be stored in designated bunded areas away from drainage lines and boreholes. | Desktop | Prior to construction | Applicant | One off | Ensure compliance with relevant legislation | No legal directives Legal compliance audit scores |
| Spill prevention and spill response measures should be implemented on site at all times. | Desktop | Prior to construction | Applicant | One off | Ensure compliance with relevant legislation | No legal directives Legal compliance audit scores |

| | | | | | | |
|---|---------|-----------------------|-----------|---------|---|--|
| Disturbed areas should be rehabilitated following completion of drilling activities. Boreholes not required for future monitoring or exploration purposes should be appropriately sealed and decommissioned in accordance with recognised standards | Desktop | Prior to construction | Applicant | One off | Ensure compliance with relevant legislation | No legal directives Legal compliance audit scores |
|---|---------|-----------------------|-----------|---------|---|--|

10 Conclusions

Based on the available desktop information, the study area is inferred to be underlain predominantly by fractured metamorphic and intrusive bedrock aquifers, in which groundwater occurrence is expected to be controlled mainly by secondary features such as fractures and shear zones. From a regional desktop perspective, the 1:500 000 scale hydrogeological map indicates that the study area is associated predominantly with fractured aquifer conditions, with localised occurrences of intergranular and fractured aquifers toward the western extent. In terms of this regional mapping, average borehole yields in the area are indicated to be in the order of 0.1 to 0.5 L/s; however, these values are map-derived regional yield indications and do not represent site-confirmed or tested borehole yields.

Recharge is likewise expected to be low. The Vegter Groundwater Recharge Map indicates recharge values in the order of 0.1 to 3 mm/a, while the GRA2 dataset indicates recharge approaching negligible values in parts of the area. These values suggest a low-recharge arid groundwater environment but should be regarded as regional screening-level estimates rather than site-specific quantified recharge.

Site-specific groundwater information remains limited. The NGA database identified 84 boreholes in and around the study area and within a 1 km radius thereof, with only three (3) groundwater level measurements and one (1) discharge value recorded. The available data indicates groundwater levels ranging from approximately 15 to 45 mbgl, recorded between 1990 and 1996. A single discharge value of approximately 2 L/s was recorded for borehole 2919BD00030.

The desktop groundwater assessment suggests that groundwater can potentially occur locally within structurally favourable zones, particularly where fracturing, shearing, or lithological contacts enhance secondary permeability. However, the current level of information is insufficient to confirm groundwater potential, sustainable yield, groundwater quality, or groundwater flow conditions on site. The present assessment should therefore be regarded as a scoping-level appraisal only, with further field-based hydrogeological investigation required to verify groundwater conditions and inform any future groundwater development or regulatory process.

Assumptions

- This assessment is based primarily on available desktop information, regional geological and hydrogeological datasets, and the proposed prospecting scope involving the drilling of approximately sixteen boreholes. No site-specific aquifer testing, groundwater quality sampling, or long-term groundwater monitoring data were available for the study area at the time of assessment. As such, several assumptions and uncertainties remain regarding the local hydrogeological conditions.

The assessment assumes that:

- The proposed activities are limited to prospecting-related drilling only and do not include mining, bulk abstraction, infrastructure development, or large-scale groundwater dewatering;
- Boreholes will be drilled using recognised industry-standard drilling methods and environmental management practices;

- Drilling fluids, fuels, and hydrocarbons will be appropriately managed onsite;
- All boreholes not required for future monitoring or exploration purposes will be sealed and rehabilitated in accordance with recognised standards.

Uncertainties remain regarding:

- The exact depth, extent, and hydraulic properties of local aquifer systems;
- Groundwater flow directions and seasonal groundwater level fluctuations;
- The potential presence of shallow perched groundwater zones or preferential flow pathways associated with geological structures;
- Existing baseline groundwater quality conditions in the immediate prospecting area.
- Due to the limited scale and temporary nature of the proposed prospecting activities, these uncertainties are not expected to materially alter the overall finding that impacts associated with the drilling programme are anticipated to be of low significance, provided mitigation measures are implemented.

Specialist Opinion:

No fatal flaws or showstoppers has been identified at the Desktop in terms of the Hydrogeological conditions at Sandgat project area.

11 Recommendations

Recommendations for Prospecting Activities

The following recommendations are specifically related to the proposed prospecting drilling activities and are intended to improve understanding of the local hydrogeological conditions while mitigating potential impacts associated with prospecting only:

- Boreholes should be positioned outside of wetlands, drainage features, and environmentally sensitive areas where reasonably possible;
- Appropriate spill prevention and containment measures must be implemented onsite during drilling activities;
- Hydrocarbons, drilling additives, and hazardous substances should be stored within designated bunded areas away from drainage features;
- Drilling fluids and cuttings should be managed and disposed of in an environmentally responsible manner;
- Disturbed areas should be rehabilitated as soon as reasonably possible following completion of drilling activities;
- Boreholes not required for future exploration or monitoring purposes should be sealed and decommissioned in accordance with recognised standards;
- Basic groundwater strike information encountered during drilling, including water levels and approximate yields, should be recorded to improve understanding of the local hydrogeological setting;
- Any visible groundwater seepages, inflows, or unexpected groundwater conditions encountered during drilling should be documented and assessed;


- If groundwater contamination, artesian conditions, or significant water strikes are encountered during drilling, additional hydrogeological assessment may be required; and
- Based on the limited scale, temporary duration, and nature of the proposed prospecting drilling programme, no significant long-term impacts on groundwater resources are anticipated.

12 References

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Signature Page

Hydrogeek Consulting (Pty) Ltd

| Authorisation | | | |
|---------------|--|--|----------|
| Name | Designation | Signature | Date |
| NL van Zyl | Project Hydrogeologist <i>Pr.Sci.Nat.</i> |  | May 2026 |

Reg.No. 2019/327216/07

Directors: NL van Zyl

Appendix 1

NGA Data within and proximal to the study are

| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|-----------|
| 2919BC00053 | -29.35453 | 19.64652 | | | | | | 13/06/1989 | 30.00 | | 74.00 | 0.00 | 30.00 | Alluvium |
| | | | | | | | | | | | | 30.00 | 84.00 | Granite |
| | | | | | | | | | | | | 0.00 | 30.00 | Alluvium |
| | | | | | | | | | | | | 30.00 | 84.00 | Granite |
| 2919BC00055 | -29.35454 | 19.64652 | | | | | | 09/06/1989 | 78.00 | 4 | 42.00 | 0.00 | 30 | Quartzite |
| | | | | | | | | | | | | 30.00 | 78 | Granite |
| 2919BC00054 | -29.35453 | 19.64653 | | | | | | 07/06/1989 | 114.00 | | | 0.00 | 114 | Granite |
| 2919BC00037 | -29.35036 | 19.64921 | | | | | | 27/01/1938 | 45.11 | | 42.67 | 0.00 | 45.11 | Quartzite |
| 2919BC00038 | -29.28369 | 19.66588 | | | | | | 08/11/1977 | 54.00 | | | 0.00 | 5 | Chalk |
| | | | | | | | | | | | | 5.00 | 12 | Gneiss |
| | | | | | | | | | | | | 12.00 | 20 | Schist |
| | | | | | | | | | | | | 20.00 | 54 | Quartzite |
| 2919BC00039 | -29.28369 | 19.66589 | | | | | | 10/11/1977 | 24.00 | | 18 | 0.00 | 24 | Quartzite |
| 2919BC00040 | -29.33369 | 19.68254 | | | | | | 05/06/1937 | 40.23 | | 32.91 | 0.00 | 40.23 | Quartzite |
| 2919BC00041 | -29.30036 | 19.72421 | | | | | | 27/10/1954 | 56.38 | 3 | 26.51 | 0.00 | 56.38 | Granite |
| 2919BC00043 | -29.30037 | 19.72421 | | | | | | 25/01/1960 | 53.34 | | 48.76 | 0.00 | 53.34 | Quartzite |
| 2919BC00045 | -29.30038 | 19.72421 | | | | | | 21/05/1967 | 77.42 | 2 | 44.8 | 0.00 | 50.9 | Quartzite |
| | | | | | | | | | | | | 50.90 | 77.42 | Granite |
| 2919BC00047 | -29.30039 | 19.72421 | | | | | | 28/03/1973 | 76.50 | | 25 | 0.00 | 27.43 | Quartzite |
| | | | | | | | | | | | | 27.43 | 76.5 | Dolerite |
| 2919BC00049 | -29.3004 | 19.72421 | | | | | | 31/08/1940 | 54.25 | | 52.73 | 0.00 | 32 | Quartzite |
| | | | | | | | | | | | | 32.00 | 36.88 | Granite |
| | | | | | | | | | | | | 36.88 | 41.45 | Dolomite |
| | | | | | | | | | | | | 41.45 | 54.25 | Granite |
| 2919BC00051 | -29.30041 | 19.72421 | | | | | | 18/05/1937 | 120.09 | | | 0.00 | 39.92 | Granite |
| | | | | | | | | | | | | 39.92 | 71.01 | Quartzite |
| | | | | | | | | | | | | 71.01 | 74.37 | Granite |
| | | | | | | | | | | | | 74.37 | 120.09 | Quartzite |
| 2919BC00042 | -29.30036 | 19.72422 | | | | | | 05/01/1960 | 167.64 | | 39.62 | 0.00 | 19.5 | Quartzite |
| | | | | | | | | | | | | 19.50 | 29.87 | Granite |
| | | | | | | | | | | | | 29.87 | 167.64 | Quartzite |
| 2919BC00044 | -29.30036 | 19.72423 | | | | | | 15/03/1960 | 50.29 | | 42.67 | 0.00 | 14.02 | Clay |
| | | | | | | | | | | | | 14.02 | 24.07 | Quartzite |
| | | | | | | | | | | | | 24.07 | 35.66 | Dolerite |
| | | | | | | | | | | | | 35.66 | 50.29 | Granite |
| 2919BC00046 | -29.30036 | 19.72424 | | | | | | 11/06/1969 | 61.26 | 18 | 32 | 0.00 | 17.06 | Shale |
| | | | | | | | | | | | | 17.06 | 36.57 | Quartzite |
| | | | | | | | | | | | | 36.57 | 61.26 | Granite |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|---------------------|--------------------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|-----------|
| 2919BC00048 | -29.30036 | 19.72425 | | | | | | 26/03/1973 | 94.18 | | 81 | 0.00 | 6.09 | Limestone |
| | | | | | | | | | | | | 6.09 | 94.18 | Quartzite |
| 2919BC00050 | -29.30036 | 19.72426 | | | | | | 06/07/1940 | 62.78 | | 54.86 | 0.00 | 6.88 | Quartzite |
| | | | | | | | | | | | | 6.88 | 62.78 | Granite |
| 2919BC00052 | -29.30036 | 19.72427 | | | | | | 01/01/1900 | 55.16 | | | 0.00 | 55.16 | Granite |
| 2919BD00032 | -29.33647 | 19.77643 | Static Water Level | 1996/04/24 08:00:00.000 | 31 | | | 24/04/1996 | 6.00 | 6 | 31 | 0.00 | 72 | Quartzite |
| | | | | | | | | | | | 56 | | | |
| | | | | | | | | | | | 31 | | | |
| | | | | | | | | | | | 56 | | | |
| 2919BD00036 | -29.28092 | 19.79559 | | | | | | 16/06/1969 | 98.76 | | 98.76 | 0 | 68.28 | Quartzite |
| | | | | | | | | | | | | 68.28 | 98.76 | Granite |
| 2919BD00037 | -29.28091 | 19.79559 | | | | | | 04/06/1969 | 107.90 | | 107.9 | 0 | 68.28 | Quartzite |
| | | | | | | | | | | | | 68.28 | 87.78 | Dolerite |
| | | | | | | | | | | | | 87.78 | 95.71 | Quartzite |
| | | | | | | | | | | | | 95.71 | 107.9 | Granite |
| 2919BD00034 | -29.28092 | 19.7956 | | | | | | 25/06/1996 | 59.74 | | 59.74 | 0 | 42.06 | Quartzite |
| | | | | | | | | | | | | 42.06 | 59.74 | Granite |
| 2919BD00035 | -29.28091 | 19.7956 | | | | | | 11/06/1969 | 83.52 | | 83.52 | 0 | 23.77 | Quartzite |
| | | | | | | | | | | | | 23.77 | 83.52 | Granite |
| 2919BB00004 | -29.21786 | 19.81504 | | | | | | 18/09/1991 | 114.00 | | | 0 | 114 | Granite |
| 2919BB00136 | -29.16703 | 19.83254 | | | | | | 11/01/1979 | 84.00 | | | 0 | 3 | SOIL |
| | | | | | | | | | | | 3 | 84 | Granite | |
| 2919BD00011 | -29.31703 | 19.86588 | | | | | | 26/12/1940 | 51.82 | | 39.01 | 0 | 51.82 | Quartzite |
| 2919BD00013 | -29.31704 | 19.86588 | | | | | | 07/11/1940 | 67.06 | | | 0 | 0.3 | Quartzite |
| | | | | | | | | | | | 0.3 | 53.95 | Granite | |
| | | | | | | | | | | | 53.95 | 67.06 | Quartzite | |
| 2919BD00015 | -29.31705 | 19.86588 | | | | | | 10/07/1967 | 120.09 | | 103.63 | 0 | 55.47 | Granite |
| | | | | | | | | | | | | 55.47 | 112.78 | Quartzite |
| | | | | | | | | | | | | 112.78 | 120.09 | Granite |
| | | | | | | | | | | | 106.68 | 0 | 55.47 | Granite |
| | | | | | | | | | | | | 55.47 | 112.78 | Quartzite |
| | | | | | | | | | | | | 112.78 | 120.09 | Granite |
| 2919BD00012 | -29.31703 | 19.86589 | | | | | | 04/02/1937 | 111.25 | | 82.3 | 0 | 111.25 | Quartzite |
| 2919BD00016 | -29.31703 | 19.86591 | | | | | | 02/06/1960 | 92.66 | | 88.39 | 0 | 92.66 | Quartzite |
| 2919BD00014 | -29.317029999999999 | 19.865900000000000 | | | | | | 28/07/1969 | 96.93 | | 73.15 | 0 | 15.24 | Shale |
| | | | | | | | | | | | | 15.24 | 96.93 | Quartzite |
| | | | | | | | | | | | | 88.39 | 0 | 15.24 |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|-----------|
| 2919BD00023 | -29.30036 | 19.86588 | | | | | | 24/03/1938 | 123.93 | | | 15.24 | 96.93 | Quartzite |
| | | | | | | | | | | | 94.49 | 0 | 15.24 | Shale |
| | | | | | | | | | | | | 15.24 | 96.93 | Quartzite |
| | | | | | | | | | | | 61.26 | 0 | 25.91 | Sandstone |
| 2919BD00023 | -29.30036 | 19.86588 | | | | | | 24/03/1938 | 123.93 | | | 25.91 | 70.71 | Shale |
| | | | | | | | | | | | | 70.71 | 103.33 | Quartzite |
| | | | | | | | | | | | | 103.33 | 110.64 | Granite |
| | | | | | | | | | | | | 110.64 | 116.74 | Quartzite |
| | | | | | | | | | | | | 116.74 | 118.57 | Shale |
| | | | | | | | | | | | | 118.57 | 123.93 | Quartzite |
| | | | | | | | | | | | | 0 | 10.36 | Quartzite |
| | | | | | | | | | | | | 10.36 | 27.74 | Quartzite |
| 2919BD00024 | -29.30037 | 19.86588 | | | | | | 09/05/1938 | 67.33 | | 29.87 | 27.74 | 35.05 | Quartzite |
| | | | | | | | | | | | | 35.05 | 36.88 | Quartzite |
| | | | | | | | | | | | | 36.88 | 46.02 | Gneiss |
| | | | | | | | | | | | | 46.02 | 47.24 | Quartzite |
| | | | | | | | | | | | | 47.24 | 67.33 | Gneiss |
| | | | | | | | | | | | | 0 | 3.05 | Limestone |
| 2919BD00026 | -29.30038 | 19.86588 | | | | | | 28/08/1946 | 107.59 | | 45.72 | 3.05 | 9.14 | Clay |
| | | | | | | | | | | | | 9.14 | 18.29 | Shale |
| | | | | | | | | | | | | 18.29 | 37.49 | Sandstone |
| | | | | | | | | | | | | 37.49 | 107.59 | Granite |
| | | | | | | | | | | | 51.82 | 0 | 3.05 | Limestone |
| | | | | | | | | | | | | 3.05 | 9.14 | Clay |
| | | | | | | | | | | | | 9.14 | 18.29 | Shale |
| | | | | | | | | | | | | 18.29 | 37.49 | Sandstone |
| | | | | | | | | | | | | 37.49 | 107.59 | Granite |
| | | | | | | | | | | | | 0 | 6.1 | Sandstone |
| 2919BD00028 | -29.30039 | 19.86588 | | | | | | 25/07/1966 | 122.22 | | 38.71 | 6.1 | 46.33 | Quartzite |
| | | | | | | | | | | | | 46.33 | 74.37 | Granite |
| | | | | | | | | | | | | 74.37 | 101.8 | Quartzite |
| | | | | | | | | | | | | 101.8 | 122.22 | Granite |
| 2919BD00025 | -29.30036 | 19.86589 | | | | | | 05/12/1946 | 114.00 | | | 0 | 10.67 | Quartzite |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|------------|
| 2919BD00027 | -29.30036 | 19.8659 | | | | | | 19/07/1966 | 107.59 | | 38.4 | 10.67 | 114 | Granite |
| | | | | | | | | | | | | 0 | 4.57 | Sand-stone |
| | | | | | | | | | | | | 4.57 | 89.61 | Quartzite |
| 2919BD00029 | -29.30036 | 19.86591 | | | | | | 29/07/1966 | 76.50 | | 52.73 | 0 | 67.67 | Quartzite |
| | | | | | | | | | | | | 67.67 | 76.5 | Granite |
| | | | | | | | | | | | | 0 | 67.67 | Quartzite |
| 2919BD00030 | -29.29619 | 19.8881 | | 1990/08/02 08:00:00.000 | 45 | 1990/08/02 00:00:00.000 | 2.000 l/s | 02/08/1990 | 6.60 | 7 | 60 | 0 | 66 | Quartzite |
| | | | | | | | | | 60.00 | | | | | |
| | | | | | | | | | | | | 67.67 | 76.5 | Granite |
| 2919BD00061 | -29.28452 | 19.89098 | | | | | | 20/03/1958 | 165.81 | | 165.81 | 0 | 165.81 | Quartzite |
| 2652 | -29.2845 | 19.89195 | | | | | | 13/07/1949 | 93.88 | | 93.88 | 0 | 2.74 | SOIL |
| | | | | | | | | | | | | 2.74 | 23.77 | Granite |
| | | | | | | | | | | | | 23.77 | 93.88 | Quartzite |
| 2804 | -29.28453 | 19.89195 | | | | | | 25/05/1949 | 93.27 | | 93.27 | 0 | 15.54 | Quartzite |
| | | | | | | | | | | | | 15.54 | 93.27 | Schist |
| 2919BD00053 | -29.28453 | 19.89196 | | | | | | 08/09/1937 | 107.90 | 7 | 98.75 | 0 | 107.9 | Quartzite |
| 2919BD00047 | -29.28455 | 19.89197 | | | | | | 13/10/1958 | 86.26 | | 86.26 | 0 | 49.38 | Quartzite |
| | | | | | | | | | | | | 49.38 | 53.64 | Granite |
| | | | | | | | | | | | | 53.64 | 57.3 | Quartzite |
| | | | | | | | | | | | | 57.3 | 86.26 | Granite |
| 2919BD00049 | -29.28453 | 19.89197 | | | | | | 13/07/1961 | 136.55 | | 125.58 | 0 | 132.89 | Quartzite |
| | | | | | | | | | | | | 132.89 | 136.55 | Granite |
| 2919BD00051 | -29.28453 | 19.89198 | | | | | | 16/02/1959 | 151.49 | | 151.49 | 0 | 58.22 | Quartzite |
| | | | | | | | | | | | | 58.22 | 69.49 | Granite |
| | | | | | | | | | | | | 69.49 | 76.5 | Quartzite |
| | | | | | | | | | | | | 76.5 | 112.17 | Granite |
| | | | | | | | | | | | | 112.17 | 129.24 | Quartzite |
| | | | | | | | | | | | | 129.24 | 151.49 | Granite |
| 2919BD00052 | -29.28451 | 19.89198 | | | | | | 15/01/1962 | 67.06 | | 67.06 | 0 | 15.24 | Quartzite |
| | | | | | | | | | | | | 15.24 | 67.06 | Granite |
| 2919BD00060 | -29.28453 | 19.89198 | | | | | | 22/08/1958 | 85.65 | | 85.65 | 0 | 85.65 | Quartzite |
| 2919BD00038 | -29.28453 | 19.89199 | | | | | | 07/09/1954 | 154.23 | 5 | 113.39 | 0 | 4.88 | Lime-stone |
| | | | | | | | | | | | | 4.88 | 32.31 | Granite |
| | | | | | | | | | | | | 32.31 | 154.23 | Quartzite |
| 2919BD00045 | -29.28452 | 19.89199 | | | | | | 30/04/1967 | 156.06 | | 156.06 | 0 | 4.57 | Quartzite |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|------------|
| | | | | | | | | | | | | 4.57 | 156.06 | Granite |
| 2919BD00048 | -29.28455 | 19.89199 | | | | | | 17/07/1958 | 80.47 | | 80.47 | 0 | 2.44 | Quartzite |
| | | | | | | | | | | 2.44 | | 36.88 | Granite | |
| | | | | | | | | | | 36.88 | | 48.16 | Quartzite | |
| | | | | | | | | | | 48.16 | | 80.47 | Granite | |
| 2919BD00050 | -29.28451 | 19.89199 | | | | | | 13/05/1954 | 34.44 | | 34.44 | 0 | 34.44 | Schist |
| 2919BD00054 | -29.2845 | 19.89199 | | | | | | 15/06/1937 | 75.29 | | 75.29 | 0 | 2.13 | SOIL |
| | | | | | | | | | | 2.13 | | 75.29 | Quartzite | |
| 2919BD00057 | -29.28449 | 19.89199 | | | | | | 28/09/1949 | 89.31 | | 89.31 | 0 | 0.3 | SOIL |
| | | | | | | | | | | 0.3 | | 1.52 | Calcrete | |
| | | | | | | | | | | 1.52 | | 8.23 | Sand-stone | |
| | | | | | | | | | | 8.23 | | 50.6 | Granite | |
| | | 19.89199 | | | | | | | | | | 50.6 | 89.31 | Quartzite |
| 2919BD00059 | -29.28454 | 19.89199 | | | | | | 19/05/1958 | 15.85 | | 15.85 | 0 | 15.85 | Quartzite |
| 2919BD00062 | -29.28452 | 19.89199 | | | | | | 01/01/1900 | 24.08 | | | 0 | 24.08 | Quartzite |
| 2919BD00063 | -29.28453 | 19.89199 | | | | | | 03/08/1961 | 28.96 | | 28.96 | 0 | 28.96 | Quartzite |
| 2919BD00040 | -29.28454 | 19.892 | | | | | | 12/11/1957 | 29.87 | | 29.87 | 0 | 29.87 | Schist |
| 2919BD00044 | -29.28453 | 19.892 | | | | | | 04/12/1961 | 155.14 | | 155.14 | 0 | 38.1 | Quartzite |
| | | | | | | | | | | 38.1 | | 155.14 | Granite | |
| 2919BD00046 | -29.28452 | 19.892 | | | | | | 02/10/1945 | 38.71 | | 38.71 | 0 | 27.74 | Quartzite |
| | | | | | | | | | | 27.74 | | 38.71 | Granite | |
| 2919BD00058 | -29.28453 | 19.892 | | | | | | 08/12/1949 | 89.61 | | 89.61 | 0 | 89.61 | Quartzite |
| 2501 | -29.28455 | 19.89301 | | | | | | 22/02/1949 | 95.71 | | 95.71 | 0 | 5.18 | Lime-stone |
| | | | | | | | | | | 5.18 | | 33.53 | Quartzite | |
| | | | | | | | | | | 33.53 | | 95.71 | Granite | |
| 2919BD00043 | -29.28456 | 19.89302 | | | | | | 23/06/1954 | 60.96 | | 60.96 | 0 | 21.34 | Schist |
| | | | | | | | | | | 21.34 | | 30.48 | Quartzite | |
| | | | | | | | | | | 30.48 | | 60.96 | Granite | |
| 2919BD00042 | -29.28457 | 19.89303 | | | | | | 05/05/1958 | 169.16 | | 169.16 | 0 | 13.72 | Quartzite |
| | | | | | | | | | | 13.72 | | 86.56 | Schist | |
| | | | | | | | | | | 86.56 | | 166.12 | Quartzite | |
| | | | | | | | | | | 166.12 | | 169.16 | Granite | |
| 2919BD00041 | -29.28458 | 19.89304 | | | | | | 02/02/1946 | 111.25 | | 93.27 | 0 | 1.52 | Calcrete |
| | | | | | | | | | | 1.52 | | 7.32 | Quartzite | |
| | | | | | | | | | | 7.32 | | 42.37 | Granite | |
| | | | | | | | | | | 42.37 | | 57.91 | Quartzite | |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|--------------|
| | | | | | | | | | | | | 57.91 | 64.01 | Granite |
| | | | | | | | | | | | | 64.01 | 67.67 | Quartzite |
| | | | | | | | | | | | | 67.67 | 72.24 | Granite |
| | | | | | | | | | | | | 72.24 | 111.25 | Quartzite |
| G01322NC | -29.21283 | 19.93116 | | | | | | | | | | | | |
| 2919BB00125 | -29.20036 | 19.94921 | | | | | | 13/10/1959 | 105.46 | | 95.7 | 0 | 105.46 | Quartzite |
| 2919BB00127 | -29.20037 | 19.94921 | | | | | | 17/12/1953 | 148.13 | | | 0 | 51.81 | Quartzite |
| | | | | | | | | | | | | 51.81 | 148.13 | Granite |
| 2919BB00129 | -29.20038 | 19.94921 | | | | | | 07/05/1934 | 116.43 | | | 0 | 9.14 | Clay |
| | | | | | | | | | | | | 9.14 | 116.43 | Quartzite |
| 2919BB00131 | -29.20039 | 19.94921 | | | | | | 12/09/1934 | 120.70 | | | 0 | 120.7 | Quartzite |
| 2919BB00133 | -29.2004 | 19.94921 | | | | | | 17/01/1935 | 62.48 | | | 0 | 32 | Vein-Quartz |
| | | | | | | | | | | | | 32 | 62.48 | Granite |
| 2919BB00135 | -29.20041 | 19.94921 | | | | | | 06/08/1912 | 60.35 | | | 0 | 60.35 | Granite |
| 2919BB00126 | -29.20036 | 19.94922 | | | | | | 31/03/1954 | 166.11 | | 146.3 | 0 | 85.95 | Quartzite |
| | | | | | | | | | | | | 85.95 | 166.11 | Granite |
| 2919BB00128 | -29.20036 | 19.94923 | | | | | | 15/10/1953 | 35.35 | | | 0 | 1.52 | Limestone |
| | | | | | | | | | | | | 1.52 | 35.35 | Quartzite |
| 2919BB00130 | -29.20036 | 19.94924 | | | | | | 25/06/1934 | 99.36 | 15 | 69.8 | 0 | 36.57 | Vein-Quartz |
| | | | | | | | | | | | | 36.57 | 99.36 | Gravel |
| 2919BB00132 | -29.20036 | 19.94925 | | | | | | 26/10/1934 | 68.27 | | | 0 | 68.27 | Quartzite |
| 2919BB00134 | -29.20036 | 19.94926 | | | | | | 04/08/1950 | 100.88 | | 67.05 | 0 | 7.62 | Limestone |
| | | | | | | | | | | | | 7.62 | 99.66 | Quartzite |
| | | 19.94926 | | | | | | | | | | 99.66 | 100.88 | Granite |
| 2919BD00031 | -29.29758 | 19.9881 | | | | | | 06/08/1990 | 6.00 | 6 | | 0 | 6 | Conglomerate |
| | | | | | | | | | | | | 6 | 90 | Quartzite |
| | | | | | | | | | 90.00 | | | 0 | 6 | Conglomerate |
| | | | | | | | | | | | | 6 | 90 | Quartzite |
| 2920AC00013 | -29.43119 | 20.00065 | | 1990/11/06 08:00:00.000 | 15 | | | 06/11/1990 | 30.00 | 6 | | 0 | 30 | Granite |
| 2920AC00011 | -29.30035 | 20.06593 | | | | | | 20/06/1985 | 18.00 | 6 | 10 | 0 | 18 | Granite |



| Identifier | Latitude | Longitude | Water Level Status | WaterLevel Measurement Date And Time | Water Level | Dis-chargeRate Measurement Date and Time | Dis-chargeRate (L/s) | Depth Diameter Measurement Date | Depth Diameter Depth to Bot | Casing Depth to Bot | Water Strike mBGL | Lithology Depth to Top | Lithology Depth to Bot | Lithology |
|-------------|-----------|-----------|--------------------|--------------------------------------|-------------|--|----------------------|---------------------------------|-----------------------------|---------------------|-------------------|------------------------|------------------------|-----------|
| 2920AC00012 | -29.30036 | 20.06593 | | | | | | 21/06/1985 | 30.00 | 6 | 24 | 0 | 30 | Granite |
| 2920AC00016 | -29.37758 | 20.09898 | | | | | | 17/04/1991 | 4.00 | 4 | | 0 | 54 | Quartzite |
| | | | | | | | | | 54.00 | | | 0 | 54 | Quartzite |
| 2920AC00015 | -29.3859 | 20.09926 | | | | | | 16/04/1991 | 54.00 | 6 | | 0 | 3 | Chalk |
| | | | | | | | | | | | | 3 | 54 | Quartzite |
| | | | | | | | | | 6.00 | | | 0 | 3 | Chalk |
| | | | | | | | | | | | | 3 | 54 | Quartzite |

Appendix 2 Curriculum Vita

**Curriculum vitae****NICO VAN ZYL**

Contact Details: nico@hydrogeekconsulting.com
 +27718773744 (Mobile)
 www.linkedin.com/in/nico-van-zyl-00b33a51 (LinkedIn)
 hydrogeekconsulting.com (Company)

Education

M.Sc. Hydrogeology, University of the Free State, Bloemfontein, SA, 2011

Honours. Hydrogeology, University of the Free State, Bloemfontein, SA, 2009

B.Sc. Geology, University of the Free State, Bloemfontein, SA, 2009

Certifications

Collaboration for Success BHP Billiton - Bayside Aluminium, 5 December 2014

Languages

Afrikaans – Fluent

English – Fluent

Hydrogeek Consulting (Pty.) Ltd. – Johannesburg**Hydrogeologist/Groundwater Modeller**

Nico is passionate about the field of Hydrogeology and have a special interest in Groundwater modelling (FEFLOW) as a tool to provide cost effective solutions to clients. We are moving more to decision support modelling , through uncertainty analysis using programs that PEST tools. There is so much uncertainty to groundwater modelling it is time we accept it and let allow the flow of information to reach the right places. We are committed to the Bayesian way of thinking, that we start with uncertainty and end with uncertainty. Information reduces uncertainty.

Our experience ranges from Interpretation of groundwater data to form Conceptual Hydrogeological Models and Numerical Groundwater Models for underground and open pit mines. In the past few years we have been busy in different regions around the world, developing Numerical Groundwater Models. Regions and countries include , USA, Australia (Pilbara region), Argentina, UAE, Germany, Peru, South Africa , African countries Zambia, DRC, Mozambique, Ghana and Guinea. The focus is on helping clients with planning and management decisions in terms of groundwater solutions (pore pressures, water supply, dewatering , cone of depressions and quality impacts from mining activities). Sectors we have worked in include mining (gold, platinum, coal and diamond) , manufacturing and government.

HG aim to solve complex issues by being well-informed on what the data is telling us. Our service will provide you with interpreted data to inform your Conceptual model and Uncertainty associated. HG consulting will always be based on cost effective solutions for your Groundwater models. HG believe in under promising and over delivering as we hold our client's long-term satisfaction in the highest regards.

Why us?
With good experience in FEFLOW a modelling software package, we are confident we can provide a service that yields trustworthy and professional results

Employment History**Hydrogeek Consulting (Pty) Ltd – Johannesburg**

Director – (2019 to Present)

Golder Associates – Johannesburg*Hydrogeologist/Groundwater Modeller and Modelling team leader (2014 to 2019)***AGES – Potchefstroom***Hydrogeologist (2012 to 2014)***University of the Free State***Researcher (2011)***PROJECT EXPERIENCE – HYDROGEOLOGY****South32: Bayside Groundwater Management Plan**

Kwa-Zulu Natal, South Africa

This Groundwater Management Plan for Bayside was developed using a detailed Source-Pathway-Receptor (SPR) assessment and simulations to assess the effectiveness of the different scenarios that Bayside are considering.

Development of a post closure Water Management Plan -Phase 2 Decant Study

Gauteng, South Africa

The decant study needed to investigate the potential for the South Deep underground workings to decant on surface based on conservative assumptions. In this report the available hydrogeological data is assessed in more detail as the results and findings are critical to the understanding of the post closure flow system at South Deep.

Western Hub Dewatering and Water Supply Project

Pilbara, Australia

Groundwater Model developed in FEFLOW package. The simulations of dewatering boreholes together with water supply scenarios to meet the demand of the mine. The mine dewatering was done with dewatering boreholes. The project also involved the impact assessment and the recovery of pit water levels post closure.

Middelburg Ferrochrome – Groundwater Modelling as part of the Contaminated Land team, Middelburg, South Africa.

Mpumalanga, South Africa

This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW). Data was interpreted and used to update the conceptual model of the site. Remediation scenarios was simulated from trenches to capture contamination downstream.

Amandelbult Groundwater Model for dewatering of the underground, Thabazimbi, South Africa.

Limpopo, South Africa

This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations was used. Dewatering of two underground mines was simulated and the post closure water management scenarios were also done as part of closure, different scenarios was simulated for the TSF's.

Platreef DFS Groundwater Model, Mokopane, Limpopo Province, South Africa.

Limpopo, South Africa

This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) for EIA purposes.

Jindal Groundwater Modelling Open Pit dewatering- KwaZulu-Natal, South Africa

Kwa-Zulu Natal, South Africa

This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations will be used.

KCC Hydrogeological Services

Kolwezi, DRC

This task involved numerical flow and solute transport modelling, developing a new groundwater model us-

| | |
|--|---|
| | ing, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations in a Dolomitic karst environment. |
| Lonmin Marikana Groundwater Model Marikana, South Africa | Developing of a Groundwater Flow Model with Contaminant Transport. |
| UMK Mine Groundwater Model Hotazel, South Africa | Developing of a Groundwater Flow Model with Contaminant Transport. For closure purposes |
| Maamba Colliery HSESMP Hydrogeological Baseline and Groundwater Model Study Maamba, Zambia | This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations will be used. |
| WASSA Golden Star Dewatering Model underground and opencast mine WASSA, Ghana | The groundwater specialist study was completed to fulfil the objectives as required by the EIA, WUL and rehabilitation and closure strategy. Works included construction of a conceptual hydrogeological model and numerical groundwater model to assess the potential impacts on groundwater quantity and quality. |
| Klipspruit Colliery Mpumalanga, South Africa | This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations will be used. |

Khutala Colliery

Mpumalanga, South Africa

This task involved numerical flow and solute transport modelling, developing a new groundwater model using, a highly sophisticated and powerful 3D finite element modelling package (FEFLOW) designed to cope with complex hydrogeological situations will be used.

Two Rivers Mine South Opencast Pit

Steelpoort, South Africa

Groundwater inputs to the overall EIA and EMPR, including, geophysical surveying, drilling, testing, groundwater quantity and quality impacts, modelling, reporting of the Two Rivers Mine South Opencast Pit.

Majuba Underground Coal Gasification

Amersfoort, South Africa

Project focussed on characterizing the hydrogeology and groundwater, including pore water pressure monitoring. The development of a conceptual hydrogeological model and numerical model to identify potential impacts on the groundwater system as result of UCG operations.

Townlands Underground Mine Project

North-West, South Africa

The groundwater specialist study was completed to fulfil the objectives as required by the water management strategy, EIA, WUL and rehabilitation and closure strategy. Works included construction of a conceptual hydrogeological model and numerical groundwater model to assess the potential impacts on groundwater quantity and quality.

Bayside Aluminium

Kwazulu-Natal, South Africa

The potential impacts from these identified sources on the groundwater system and migration of contaminant plumes from these sources were evaluated through the groundwater modelling.

KCM Nchanga SPR Model Study Groundwater Flow and Contaminant Transport Modelling

Developing of a Groundwater Flow Model with Contaminant Transport.

Nchanga, Zambia

Mopani Nkana Mine Dewatering Groundwater Model

Developing of a Groundwater Flow Model to determine dewatering rates and pore pressures.

Ndola, Zambia

Mopani Mufulira Mine Dewatering Groundwater Model

Developing of a Groundwater Flow Model to determine dewatering rates and pore pressures.

Mufulira, Zambia

Kinsevere Mine Dewatering and Post Closure Groundwater Model

Developing of a Groundwater Flow Model to determine dewatering rates and impact. Post closure study to determine Pit Lake conditions.

Lubumbashi, DRC

Reminex Mines TriK Dewatering Modelling Study, Guinea 2023

Details to follow

Waterval Smelter Numerical Model , Rustenburg, North Wes 2022

Details to follow

Talon Metals Hydrogeology and Modelling Review, Minnesota, US 2022

Details to follow

Bel Air Alufer Well field design DFS Groundwater Model, Bel Air, Guinea 2022

Numerical Modelling EÜ Magdeburg Concepts for dewatering of systems, Magdeburg, Germany

Details to follow

Modelo 3D Waste Dump Las Bambas, Peru , Details to follow
South America

Barberton Mines Geohydrological Study and Details to follow
Groundwater Model, Barberton, South Africa

Bunker Hill Mine Conceptual Understanding ,
Kellogg, IDAHO, USA Details to follow

Groundwater flow and solute transport model,
San Lorenzo, Argentina Details to follow

Ardmore Graphite Open Pit Stage 3 Dewatering
Model, Queensland, Australia Details to follow

Casa Berardi Mine Principle Open Pit dewater-
ing, Quebec, Canada Details to follow

Sunnyside Diamonds Hydrogeological Concep- Details to follow
tual Model, Northern Cape, South Africa

Details to follow

Details to follow

Training

FEFLOW 7 Groundwater Modelling for Mine Sites

DHI WASY, 21-23 September 2016

FEFLOW Groundwater Modelling for Mine Sites

DHI WASY, 24 August - 26 August 2015

Advanced FEFLOW Groundwater modelling

DHI WASY, 30 October 2012- 2 November 2012

Symple School of Hydrogeological Modelling ,

Milan Italy (18 months)

SUPPLEMENTAL SKILLS

Leapfrog Hydro

Hydrogeological Modelling

Data interpretation

Surfer, GRAPHER

Maps

QGIS, Global Mapper

Groundwater Modelling

FEFLOW 8.0 and FEPEST

Python Coding**Past Statistics****PROFESSIONAL AFFILIATIONS**

Pr.Sci.Nat 400165/14

PUBLICATIONS**Conference Proceedings**

Love*, David, Vanessa Aphane, Gerhard van der Linde and Nico van Zyl. 2015. *Innovative approaches to predicting, mitigating and remediating groundwater pollution in UCG*. SAUCGA Workshop, September. Muldersdrift, South Africa.

